

# **Task 4: Problem and Solution Identification and Prioritization for Strawberry Run, Alexandria, Virginia**

Prepared for  
**City of Alexandria**  
**Transportation and Engineering Services**

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# Executive Summary

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The City of Alexandria, Virginia (the City) has experienced repeated and increasingly frequent flooding events attributable to old infrastructure, inconsistent design criteria, and perhaps climate change. The purpose of the stormwater capacity analysis project is to provide a program for analyzing storm sewer capacity issues, identifying problem areas, developing and prioritizing solutions, and providing support for public outreach and education. The project is being implemented in phases by watershed. The watersheds include Hooffs Run, Four Mile Run, Holmes Run, Cameron Run, Taylor Run, Strawberry Run, Potomac River, and Backlick Run.

This report focuses on problem and solution identification (Task 4) for capacity issues in the Strawberry Run watershed. It summarizes the problem identification steps, solution development, solution scoring, and alternatives analysis. This task has resulted in three watershed-wide alternatives aimed at resolving capacity-related problems in Strawberry Run. Additionally, this task has provided the City with a decision making process for evaluating the benefits of potential stormwater management (SWM) projects.

In Strawberry Run, the existing intensity-duration-frequency (IDF) design hyetograph for the 10-year return period, based on peak intensity, was used to simulate rainfall runoff and stormwater flow within the watershed.

The objectives of this phase of the study were to 1) identify and prioritize capacity problems based on modeling results from Task 2 of this project, and 2) develop and prioritize solutions to address those problems. The first objective was accomplished in two steps. The first step included evaluating each stormwater junction in the drainage network using a scoring system to identify problems based on several criteria, including the severity of flooding, proximity to critical infrastructure and roadways, identification of problems by City staff and the public, and opportunity for overland relief. In the next step of this objective, high-scoring junctions (that is, higher-priority problems) were grouped together to form high-priority problem areas. In total, three high-priority problem areas were identified in the Strawberry Run watershed. Flooding locations falling outside of the high-priority problem areas were either flooding at isolated structures, or did not score high on the problem identification scoring criteria. These flooding problems were not addressed by solutions in this project.

The second objective involved developing and prioritizing solutions to address capacity limitations within the three high-priority problem areas. To accomplish this objective, several strategies involving different technologies were examined, including improving conveyance by increasing hydraulic capacity, reducing capacity limitations by adding distributed storage to the system, and reducing stormwater inflows by implementing green infrastructure (GI). Each of these strategies required a different modeling approach. Conveyance improvements were modeled by increasing pipe diameter in key locations within the problem area, storage was added as storage nodes based on a preliminary siting exercise, and GI was modeled as a reduction in impervious area at three different implementation levels: high, medium, and low. A single model run was set up and run for each strategy addressing all three high-priority problem areas and the results were compiled for the alternatives and prioritization evaluation. Solutions were evaluated based on several criteria, including drainage improvement/flood reduction, environmental compliance, sustainability and social benefits, asset management and maintenance implications, constructability, and public acceptance. Planning-level capital costs were developed for each solution to facilitate a benefit cost analysis and prioritization process.

The results of the solution identification and prioritization analysis show the following results for Strawberry Run:

- In terms of solution technology performance:
  - High and Medium GI solutions generally have the greatest overall benefit in Strawberry Run.
  - Conveyance, Storage, High GI, and Medium GI solutions all provide significant flood reduction for the problem areas analyzed.
- In terms of costs:
  - A low level of GI implementation generally has the greatest benefit/cost score, but did not usually meet the minimum threshold for flood reduction.

- The cost per gallon of flood reduction appears to be highly dependent on the problem area, but in general, Conveyance, Storage, and Low GI projects provide the most economical stormwater volume reduction in terms of dollars per gallon of flood reduction within a high-priority problem area.

Three watershed-wide alternatives were developed, including:

- Alternative 1: Most cost-effective solution for each problem area (lowest dollar-per-gallon of flood reduction)
- Alternative 2: Best benefit/cost ratio for each problem area (highest benefit/cost ratio)
- Alternative 3: Combination of best projects to address the worst problem areas to the extent practicable

In Strawberry Run, the three problem areas are well spread out across the watershed and discharge to separate outfalls. For this reason, increasing the sewer pipe capacity to alleviate flooding in one problem area did not increase the flooding in other problem areas. However, the flood volumes in the problem areas are small enough that storage and GI solution projects were able to eliminate the flooding at lower cost than conveyance projects. For this reason, storage and GI projects make up the majority of projects in the watershed-wide scenarios analyzed in this study.

A summary of the watershed-wide alternatives results is provided in Table ES-1.

TABLE ES-1  
Watershed-wide Alternatives Scoring and Prioritization Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Alternative 1 - Best Cost Efficiency	Alternative 2 Best Benefit/Cost Ratio	Alternative 3 – Highest-priority Problems
Total Cost (\$ Millions)	\$0.55	\$0.62	\$0.27
Total Benefit Score	90.4	143.7	88.2
Overall Benefit/Cost	164.4	231.8	321.7
Total Flood Reduction (million gallons)	0.064	0.042	0.047
\$/Gallon of Flood Reduction	\$8.58	\$14.76	\$5.77

The three watershed-wide alternatives range in total cost from \$270,000 to \$620,000 and eliminate between 57 and 86 percent of flooding in the high-priority problem areas. Alternative 1 eliminates the most flooding of the three watershed-wide alternatives, but has the lowest benefit/cost ratio of the three alternatives. Alternative 2 has the highest benefit score, but also has the highest cost and cost/gallon of flood reduction. Alternative 3, while only addressing two of the three high-priority problem areas, has the lowest cost and cost/gallon of flood reduction and highest benefit cost ratio while eliminating 70 percent of flooding in the three high-priority problem areas and 88 percent of flooding in problem areas 601 and 602. For those reasons, Alternative 3 is the recommended alternative for the Strawberry Run watershed. Model results for the existing conditions model and the Alternative 3 watershed-wide alternative are presented in Figures ES-1 and ES-2.



FIGURE ES-1  
Existing Conditions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

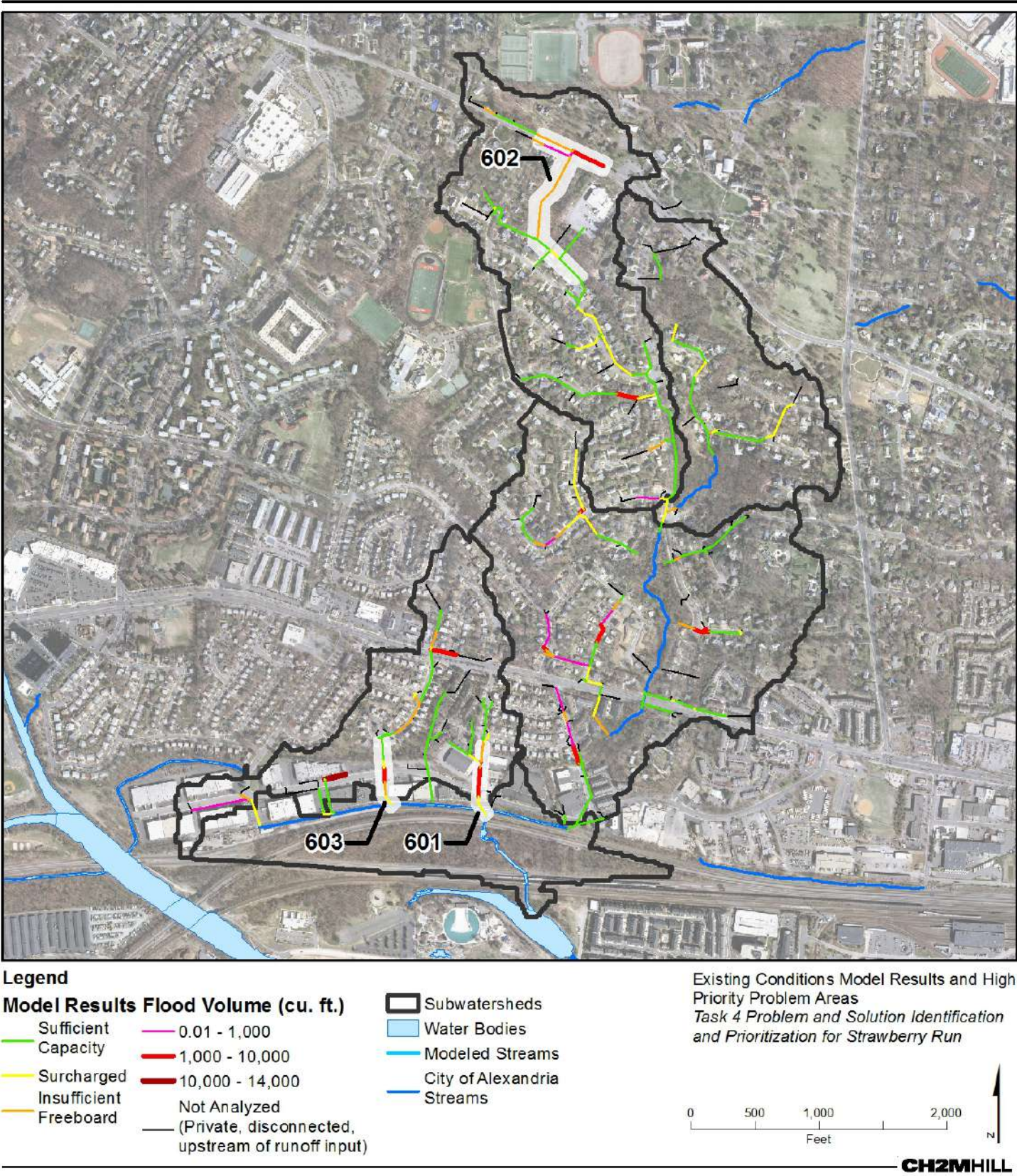
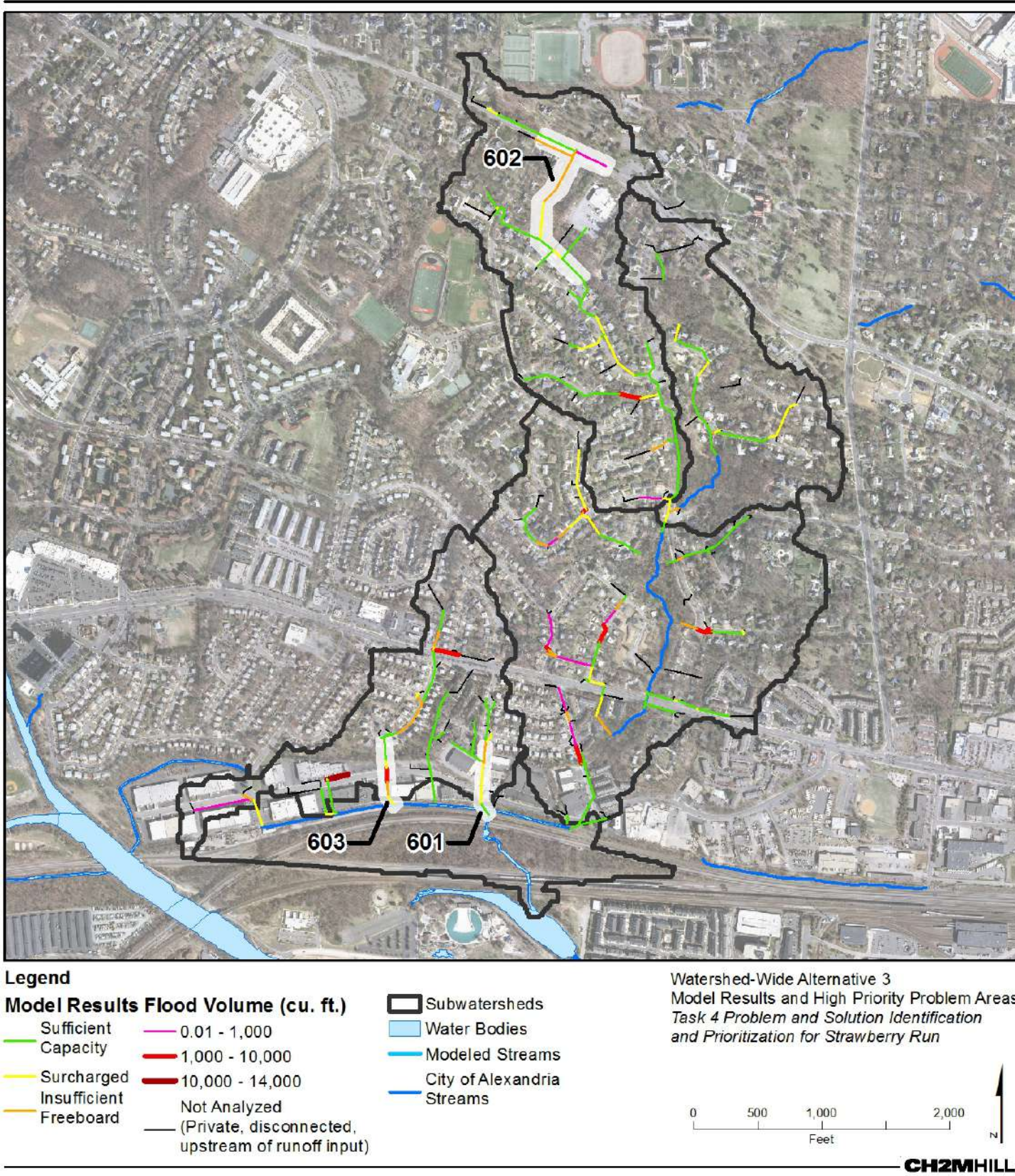


FIGURE ES-2  
Alternative 3: Highest-priority Problem Areas Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*







When developing a capital improvement plan, the benefit cost or cost efficiency (\$/gallon of flood reduction) are typically used to guide the order in which projects are implemented. Prioritization results for Alternative 3 are presented in Figure ES-3. The top chart shows the total benefit score and the cumulative capital cost of the alternative. The solutions are provided in order of decreasing benefit cost ratio; solutions with the greatest benefit cost are presented on the left and solutions with the lowest benefit cost are presented on the right. The bottom chart shows the benefit/cost ratio for each solution in the watershed-wide alternative in order of increasing cost/gallon of flood reduction. Both charts show the cumulative capital cost plotted on the secondary axis. The solutions on both charts are named by the technology: Conveyance (CONV), Storage (STOR), Low Green Infrastructure (LGI), Medium GI (MGI), or High GI (HGI), and the problem area number.

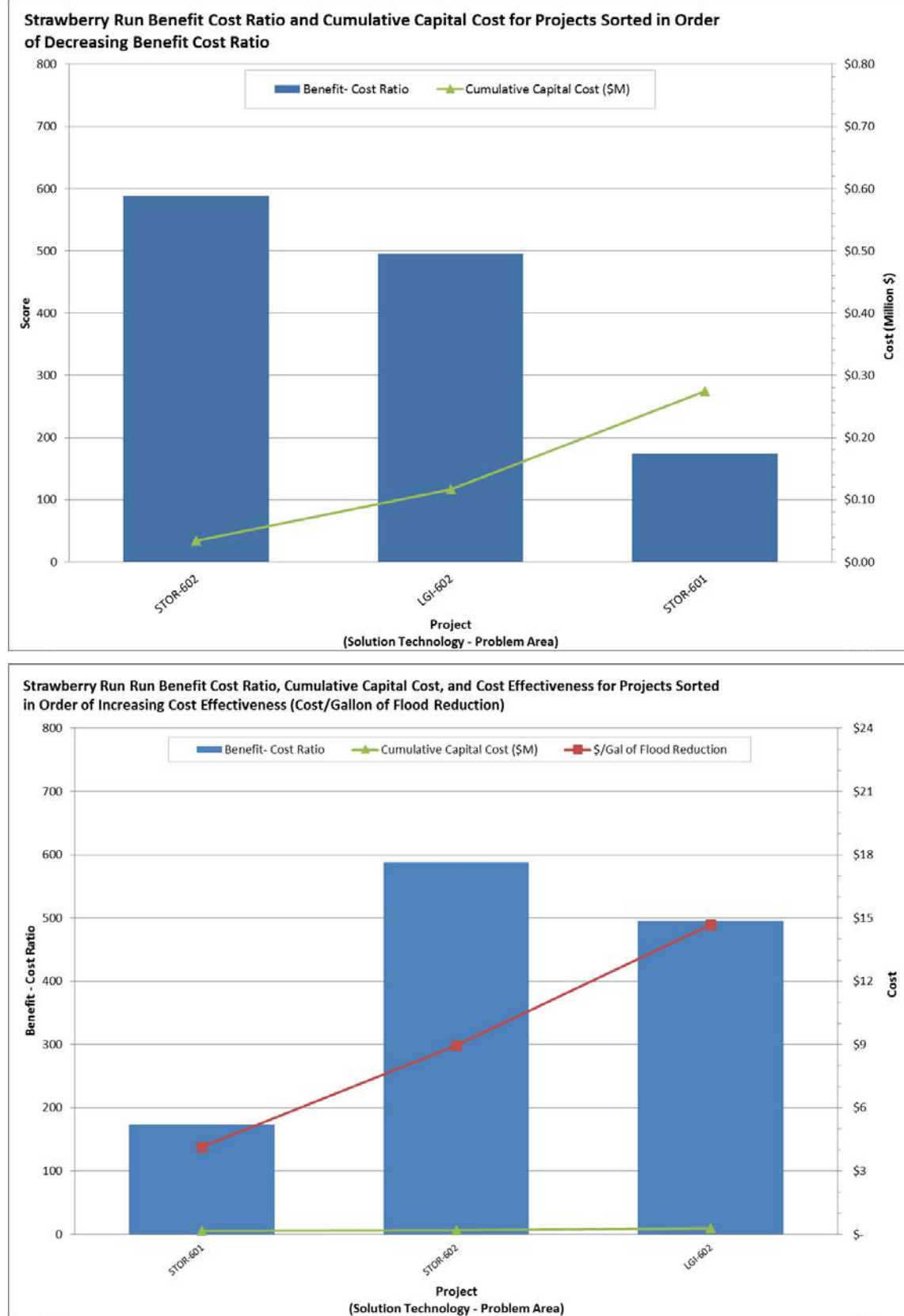
It should be noted that the model does not include analysis on private property, but applies assumed runoff loads as inputs to the public conveyance system. The City chose not to include existing private or public stormwater management facilities upstream of the modeled collection system because of the limited available information on these facilities and a concern that the facilities may not be performing as designed. When the City moves forward into detailed evaluation and design of selected projects, it will be important to fully evaluate and account for the benefits of any existing stormwater management facilities.

The hydraulic modeling results and costs presented in this report should be reviewed with the understanding that several assumptions were made to fill data gaps in the hydraulic model, and proposed solutions and costs were developed on a planning level.





FIGURE ES-3  
 Alternative 3: Highest-priority Problems Prioritization Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





# Contents

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<b>Executive Summary.....</b>	<b>iii</b>
<b>Acronyms and Abbreviations.....</b>	<b>xiii</b>
<b>1 Introduction.....</b>	<b>1-1</b>
1.1 Background.....	1-1
1.2 Objectives .....	1-1
<b>2 Approach.....</b>	<b>2-1</b>
2.1 Prioritization .....	2-1
2.1.1 Problem Area Evaluation .....	2-2
2.1.2 Solution Evaluation.....	2-3
2.2 Modeling.....	2-4
2.2.2 Baseline Improvements and Major Capacity Solutions.....	2-7
2.2.3 Alternative Solutions .....	2-7
<b>3 Problem Identification.....</b>	<b>3-1</b>
<b>4 Solution Identification.....</b>	<b>4-1</b>
4.1 Conveyance Solutions.....	4-1
4.2 Storage Solutions.....	4-6
4.3 Green Infrastructure Solutions .....	4-12
<b>5 Alternatives Analysis and Prioritization.....</b>	<b>5-1</b>
5.1 Problem Area Benefit Analysis .....	5-1
5.2 Problem Area Solution Costs .....	5-5
5.3 Problem Area Benefit/Cost Results .....	5-5
5.4 Watershed-wide Alternatives.....	5-9
5.4.1 Alternative 1: Cost Efficiency.....	5-9
5.4.2 Alternative 2: Benefit/Cost.....	5-9
5.4.3 Alternative 3: Highest-priority Problems.....	5-10
5.4.4 Modeling Results .....	5-11
5.4.5 Scoring and Prioritization Results.....	5-19
<b>6 Summary .....</b>	<b>6-1</b>
<b>7 References.....</b>	<b>7-1</b>
<b>Appendixes</b>	
A	Conveyance Solutions
B	Storage Solutions
C	Green Infrastructure Concept Plans
D	Alternatives Analysis Results
E	Basis of Cost
<b>Tables</b>	
ES-1	Watershed-wide Alternatives Scoring and Prioritization Results..... iv
2-1	Problem Area Evaluation Criteria and Weights.....2-2
2-2	Solution Evaluation Criteria and Weights.....2-3
2-3	Summary of Task 2 Model Results.....2-4
2-4	Description of Solution Modeling Approaches and Assumptions .....

3-1	Strawberry Run Problem ID Scores.....	3-1
4-1	Summary of Conveyance Projects .....	4-1
4-2	Summary of Existing Conditions and Conveyance Solutions Model Results .....	4-5
4-3	Conveyance Solutions Model Results by Problem Area .....	4-5
4-4	Storage Solutions Summary.....	4-6
4-5	Summary of Existing Conditions and Storage Solutions Model Results .....	4-11
4-6	Storage Solutions Model Results by Problem Area .....	4-11
4-7	Green Infrastructure Solutions Summary.....	4-13
4-8	Summary of Existing Conditions and Green Infrastructure Implementation Model Results .....	4-23
4-9	Green Infrastructure Solutions Flood Volume Model Results by Problem Area .....	4-24
4-10	Green Infrastructure Solutions Peak Flow Model Results by Problem Area .....	4-24
5-1	Strawberry Run Green Infrastructure Construction Costs .....	5-5
5-2	Capital Costs for High-priority Problem Area Solutions .....	5-5
5-3	Selected Projects for Watershed-wide Alternative 1: Cost Efficiency.....	5-9
5-4	Selected Projects for Watershed-wide Alternative 2: Benefit/Cost.....	5-10
5-5	Selected Projects for Watershed-wide Alternative 3: Highest-priority Problems.....	5-10
5-6	Summary of Watershed-wide Alternative Model Results .....	5-12
5-7	Watershed-wide Alternatives Scoring and Prioritization Results.....	5-19
6-1	Watershed-wide Alternatives Scoring and Prioritization Results.....	6-2

## Figures

ES-1	Existing Conditions Model Results and High-priority Problem Areas.....	v
ES-2	Alternative 3: Highest-priority Problem Areas Model Results .....	v
ES-3	Alternative 3: Highest-priority Problems Prioritization Results .....	ix
1-1	Stormwater Drainage Watersheds, City of Alexandria, Virginia.....	1-3
2-1	Revised Task 2 Existing Conditions Model Results .....	2-5
3-1	Strawberry Run Problem Identification Score Results .....	3-3
3-2	Location of Strawberry Run High-priority Problem Areas .....	3-5
4-1	Existing Conditions Model Results and High-priority Problem Areas.....	4-3
4-2	Conveyance Solutions Model Results and High-priority Problem Areas.....	4-3
4-3	Storage Solution Locations and High-priority Problem Areas .....	4-7
4-4	Storage Solutions Model Results and High-priority Problem Areas .....	4-9
4-5	Green Infrastructure Drainage Areas and High-priority Problem Areas .....	4-15
4-6	Low-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas ...	4-17
4-7	Medium-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas .....	4-19
4-8	High-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas ..	4-21
5-1	Total Benefit Score Chart for High-priority Problem Areas 601 through 603 .....	5-3
5-2	Benefit/Cost Chart for High-priority Problem Areas 601 through 603.....	5-7
5-3	Alternative 1: Cost-efficiency Model Results.....	5-13
5-4	Alternative 2: Benefit/Cost Model Results .....	5-15
5-5	Alternative 3: Highest-priority Problem Areas Model Results .....	5-17
5-6	Alternative 1: Best Cost Efficiency Prioritization Results.....	5-21
5-7	Alternative 2: Best Benefit/Cost Ratio Prioritization Results .....	5-23
5-8	Alternative 3: Highest-priority Problems Prioritization Results .....	5-25
6-1	Alternative 3: Highest-priority Problems Prioritization Results .....	6-3

# Acronyms and Abbreviations

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bgs	below ground surface
cfs	cubic feet per second
City	City of Alexandria, Virginia
ft <sup>2</sup>	square feet
ft <sup>3</sup>	cubic feet
GI	green infrastructure
HGI	high green infrastructure
HGL	hydraulic grade line
hrs	hours
ID	identification
IDF	intensity-duration-frequency
LF	linear feet
LGI	low green infrastructure
MG	million gallons
MGI	medium green infrastructure
ROW	right-of-way
SWM	stormwater management
TM	technical memorandum





# Introduction

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The City of Alexandria, Virginia (the City) has experienced repeated and increasingly frequent flooding events attributable to old infrastructure, inconsistent design criteria, and perhaps climate change. The purpose of the stormwater capacity analysis project is to provide a program for analyzing storm sewer capacity issues, identifying problem areas, developing and prioritizing solutions, and providing support for public outreach and education. The project is being implemented on a watershed basis, with Strawberry Run being the subject of this report. City of Alexandria watersheds are shown on Figure 1-1.

## 1.1 Background

The project consists of four major subtasks related to the model development and modeling. These four tasks and related technical memorandums (TMs) are described as follows:

- Task 1 – Review and propose revisions to the City’s stormwater design criteria.
  - *Updated Precipitation Frequency Results and Synthesis of New IDF Curves for the City of Alexandria, Virginia* (CH2M HILL, 2009a)
  - *Sea Level Rise Potential for the City of Alexandria, Virginia* (CH2M HILL, 2009b)
  - *Rainfall Frequency and Global Change Model Options for the City of Alexandria* (CH2M HILL, 2011)
- Task 2 – Analyze the City’s stormwater collection system capacity.
  - *Inlet Capacity Analysis for City of Alexandria Storm Sewer Capacity Analysis* (CH2M HILL, 2012)
  - *Stormwater Capacity Analysis for Strawberry Run Watershed, City of Alexandria, Virginia* (CH2M HILL, 2016a)
- Task 3 – Survey collection system facilities on pipes 24 inches and larger, to fill data gaps.<sup>1</sup>
  - *City of Alexandria Storm Sewer Capacity Analysis (CASSCA) Strawberry Run Sewershed Condition Assessment* (Baker, 2014)
- Task 4 – Identify problem areas and suggest solutions.
  - *Task 4 Evaluation Criteria Scoring Systems* (CH2M HILL, 2014)

## 1.2 Objectives

Tasks 1 through 3 focused on model development and capacity analysis of the existing system. The purpose of Task 4 is to identify and prioritize problems modeled during the Task 2 capacity analysis and to suggest and prioritize conveyance, storage, and green infrastructure (GI) solutions to resolve the identified capacity limitations.

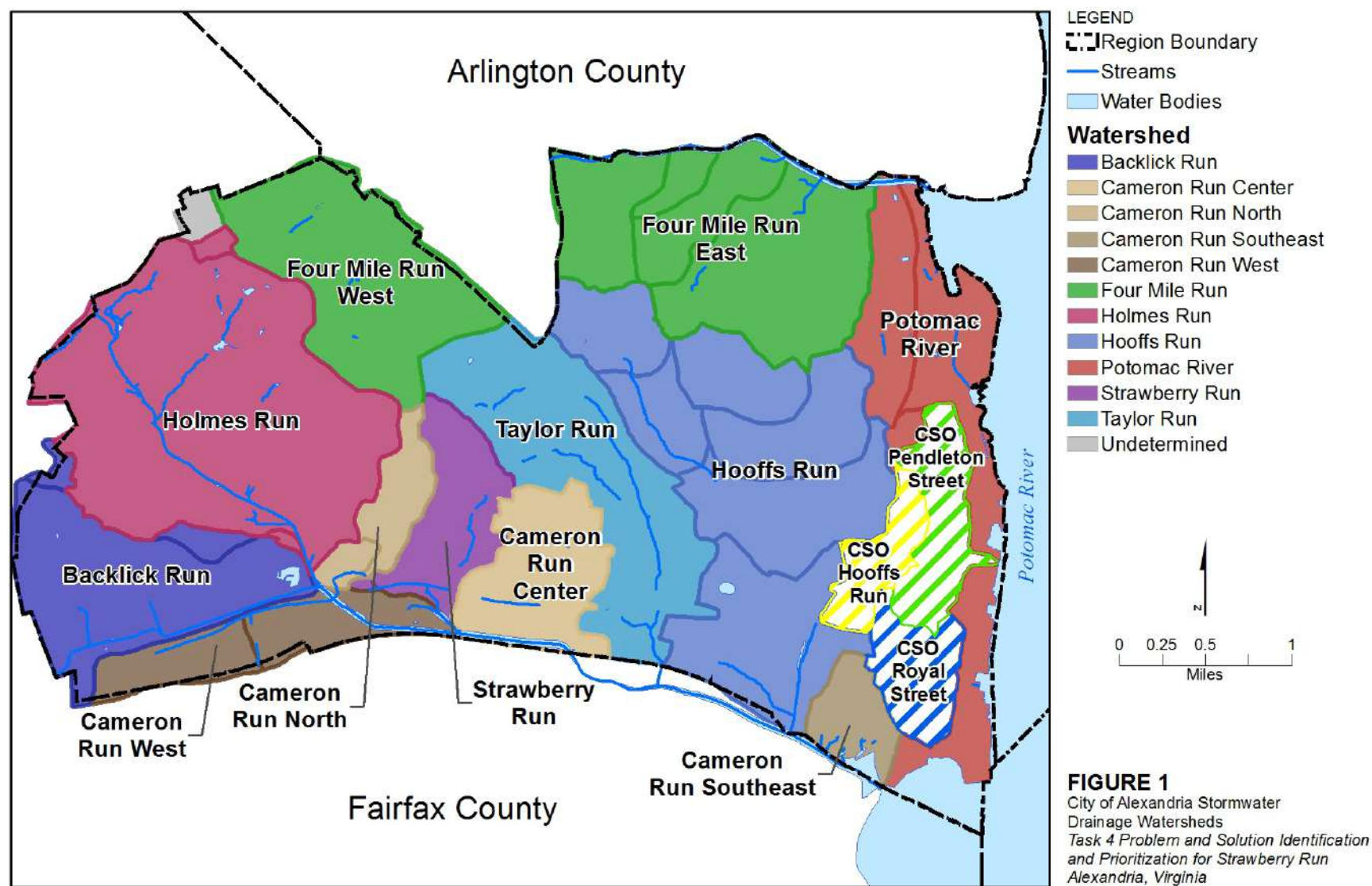
This report describes the methodology and results of Task 4 for the stormwater collection system in the Strawberry Run watershed. Figure 1-1 shows the City’s stormwater drainage watersheds.

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<sup>1</sup> Although originally intended to improve data quality where the model predicted capacity limitations, the scope of Task 3 was expanded, and field surveys were completed prior to Task 2 to fill data gaps and to improve the model development process.



FIGURE 1-1  
 Stormwater Drainage Watersheds, City of Alexandria, Virginia  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*



CH2MHILL



# Approach

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The approach to identifying and prioritizing problems and solutions included several distinct steps: identifying and prioritizing problems, developing and modeling solutions, prioritizing solutions and, finally, developing watershed-wide scenarios. This approach, described in this section, is broken into two major components: prioritization and modeling.

## 2.1 Prioritization

The focus of Task 4 is to prioritize problem areas based on Task 2 modeling results, develop solutions to resolve the problem areas, then prioritize those solutions. Before beginning the Task 4 analysis, City staff and consultants from CH2M HILL and Michael Baker convened in a workshop on November 14, 2012 to discuss the objectives, approach, and desired outcomes of this phase of the project. The major objectives of the workshop were to define the prioritization process, identify the key evaluation criteria for scoring and ranking problems and solutions, and define relative criteria weights. The prioritization process is similar for both problems and solutions, and includes several distinct steps as follows:

- **Define evaluation criteria:** Evaluation criteria for problems and solutions were defined during the Task 4 workshop with input from City staff from the Engineering & Design, Office of Environmental Quality, and Maintenance Divisions of Transportation and Engineering Services. These criteria, which are summarized in this report, were used to assess the severity of problems and the benefit of solutions.
- **Weight evaluation criteria:** Each evaluation criterion was assigned a weight (0 to 100) by Task 4 workshop participants. The weights quantify the relative importance of each evaluation criteria and build a defensible foundation for problem and solution ranking.
- **Define scoring system:** A scoring system was developed for each evaluation criteria. This provided a method for ranking problems and solutions within evaluation criteria. Scoring systems for problem area and solution evaluation criteria are defined in this report.
- **Score and rank alternatives:** Problems and solutions were scored and ranked using the evaluation criteria scoring systems, which are described in the TM entitled *Task 4 Evaluation Criteria Scoring Systems* (CH2M HILL, 2014) and include:
  - *Score and Rank Problems:* A score of 0 through 10 was assigned to stormwater junctions in the modeled system for each evaluation criteria. Weights were then applied to the score calculated for each evaluation criteria to come up with an overall weighted score for each junction. The overall score was used to rank problems; then, high-priority problem areas were identified as groupings of hydraulically-connected junctions and pipes. Solutions were investigated for the highest-priority problem areas.
  - *Score and Rank Solutions:* Solutions were developed for high-priority problem areas identified in the previous step. A score of 0 through 10 was assigned to solutions for each evaluation criteria. Then the weights were applied to the score calculated for each evaluation criteria to calculate an overall weighted benefit score. Solutions were ranked based on the overall score as well as the benefit/cost score, which is the overall benefit score divided by the capital cost of the solution. The solution evaluation is presented at the end of this report.
- **Perform “what-if” analysis to refine process:** After completing the prioritization, the process was examined to be sure the results met the expectations of the City. The result of this step was the inclusion of a 22 percent minimum threshold for flood reduction (any project that produced less than 22 percent reduction in volume of flooding was eliminated) to help focus the solution identification process. This threshold was selected by City staff based on best engineering judgment.

- **Evaluate watershed-wide scenarios:** Once individual solutions were evaluated, the solutions were grouped into three alternative watershed-wide scenarios. The scenarios were scored by summing scores and costs of individual projects for comparison. The purpose of taking this watershed-wide look at solution sets was to evaluate the solutions in a holistic, system-wide manner to evaluate the composite effects of implementing various solutions across the system and to support selection of a set of solutions that will provide the greatest benefit for the least cost.

### 2.1.1 Problem Area Evaluation

Strawberry Run watershed has a drainage area of 0.54 square miles and is located in center of the City, bounded by Cameron Run and Taylor Run watersheds. The watershed is drained by Strawberry Run and its tributaries from north to south and discharges in to Cameron Run south of Eisenhower Avenue near Cameron Parke Court. The Strawberry Run storm sewer system is made up of smaller systems that discharge along the Strawberry Run stream from north to south. As such, the effects of capacity issues are localized and do not extend throughout the system.

The problem area evaluation focused on identifying flooding problems that are extreme and/or in proximity to critical facilities. Although model results were presented for pipes, not junctions, in the Stormwater Capacity Analysis (Task 2), flooding occurs at a junction and not along the length of the pipe; therefore, stormwater junctions in the hydraulic model, not pipe segments, were scored for each of the problem area evaluation criteria. Raw scores for each criterion ranged from 0 to 10: 0 indicating the junction is not a priority and/or the evaluation criteria is not applicable, and 10 indicating the junction is a high priority. The problem area evaluation criteria includes the following:

- Urban drainage/flooding
- Identification of problems by the public
- Identification of problems by City staff
- Proximity to critical infrastructure
- Proximity to critical roadways
- Opportunity for overland relief

Detailed descriptions of the problem scoring systems used in this evaluation are provided in the TM entitled *Task 4 Evaluation Criteria Scoring Systems* (CH2M HILL, 2014). The weighted score was computed using the raw score and normalized percent weight. Evaluation criteria and weights developed and agreed upon during the Task 4 workshop are presented in Table 2-1.

TABLE 2-1  
Problem Area Evaluation Criteria and Weights  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area Evaluation Criteria	Weight	Normalized % Weight
Urban Drainage/Flooding	90	23.1
Public ID of Problem	73	18.8
City Staff ID of Problem	75	19.3
Proximity to Critical Infrastructure	58	14.9
Proximity to Critical Roadways	38	9.8
Opportunity for Overland Relief	55	14.1
<b>Total</b>	<b>389</b>	<b>100</b>

Note:  
ID - identification



After computing the weighted score for each junction, high-priority problem areas were identified as hydraulically connected groupings of junctions and pipes for the junctions with scores over 30. Scoring was based on results from the Task 2 model of the 10-year, 24-hour storm generated using the existing intensity-duration-frequency (IDF). The results of the problem area evaluation are presented in Section 3, Problem Identification, of this report.

The goal of delineating high-priority problem areas was to identify groupings of stormwater pipes causing capacity limitations so that conveyance, storage, and GI solutions could be developed for the area. This task was accomplished by starting with the highest-ranked junction score, which indicated it was the worst problem based on the problem area identification evaluation criteria, and reviewing the surrounding drainage network and model results to identify the pipes and junctions related to that high problem score. A polygon surrounding all the pipes related to the capacity limitation was digitized in ArcMap and was assigned a unique identifier. After completing this process for the highest-ranked junction score, the network and model results for the next-highest score were examined, and a new problem area was digitized. If the next highest-score was captured in the first high-priority area, it was skipped. This process was repeated for junctions with a score above 30, or the top 5.2 percent of junctions with a score over 0. Flooding occurring outside of the high-priority problem areas was either isolated or was not prioritized based on the scoring criteria. These flooding problems were not addressed by solutions developed in this analysis.

### 2.1.2 Solution Evaluation

Solutions were developed to resolve or improve capacity limitations in the highest-priority problem areas. Three different technologies were evaluated: conveyance, storage, and GI. Modeling results, described in detail in the following sections, were used in conjunction with additional data from the City (for example, geospatial data on roads and critical infrastructure, capital improvement plans, maintenance plans) to score solutions for each of the following solution evaluation criteria:

- Urban drainage/flooding
- Environmental compliance
- EcoCity goals/sustainability
- Social benefits
- Integrated asset management
- City-wide maintenance implications
- Constructability
- Public acceptability

Detailed descriptions of the solution scoring systems used in this evaluation are provided in the TM entitled *Task 4 Evaluation Criteria Scoring Systems* (CH2M HILL, 2014). The weighted score was computed using the raw score and normalized percent weight. Evaluation criteria and weights agreed upon during the Task 4 workshop are presented in Table 2-2.

TABLE 2-2  
Solution Evaluation Criteria and Weights  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Solution Evaluation Criteria	Weight	Normalized % Weight
Urban Drainage/Flooding	95	17.1
Environmental Compliance	93	16.8
EcoCity Goals/Sustainability	50	9.0
Social Benefits	40	7.2
Integrated Asset Management	73	13.2
City-wide Maintenance Implications	90	16.2
Constructability	60	10.8
Public Acceptability	53	9.6
<b>Total</b>	<b>554</b>	<b>100</b>

## 2.2 Modeling

To support the Task 4 analysis, the Strawberry Run watershed capacity was analyzed using commercially available and public domain computer models that are both widely used and industry-accepted. The details of the hydrologic and hydraulic modeling are documented in the Task 2 TM, *Stormwater Capacity Analysis for Strawberry Run Watershed, City of Alexandria, Virginia* (CH2M HILL, 2016a). The existing conditions model of the 10-year, 24-hour design storm based on the City's existing IDF curve served as the basis for modeling in the Task 4 analysis. Modifications were made to the Task 2 model to include outlet controls for known storage facilities before evaluating potential solutions. These model refinements are detailed in the following section.

### 2.2.1 Task 2 Model Refinements

Survey data and city map documents show three large diameter storage pipes in the Cameron Knoll development, located near intersection of Duke Street and South Floyd Street. These three storage pipes were included in the original Task 2 hydraulic model but the system was modeled without the outlet controls because of limited available information. Runoff input locations were chosen at the downstream end of 2 of the 3 storage pipes to avoid undue influence of the storage pipes on the model results and still fulfill the scope of the Task 2 model, which called for modeling 20 percent of inlets in the watershed. A recommendation was made to complete more detailed modeling of this area before developing projects for this portion of the storm sewer system.

During the Problem Area Evaluation step described in Section 2.1.1, the storm sewer system on South Floyd Street downstream of the Cameron Knoll development was identified as a problem area. In order to verify that this portion of the system was in fact a problem as defined by the scoring system developed for this project, and to more accurately develop solutions, additional information on the storage facilities was provided by the City. The City provided as-built drawings of the Cameron Knoll development, which included storage pipe profiles and orifice calculations for each of the three storage pipes. The size of the storage pipes was verified or corrected, outlet controls were added as round-bottom orifices, either 6 inches or 7 inches in diameter depending on the storage pipe size, and the hydrologic model was refined to route runoff inputs through the storage facilities.

Table 2-3 and Figure 2-1 present the revised Task 2 model results based on the refinements previously described.

TABLE 2-3  
Summary of Task 2 Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Existing Conditions Results			
	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>
Sufficient Capacity	13,802	54	-	-
Surcharged <sup>a</sup>	4,308	17	35	-
Insufficient Freeboard	3,392	13	-	-
Flooded	3,877	15	13	67,657

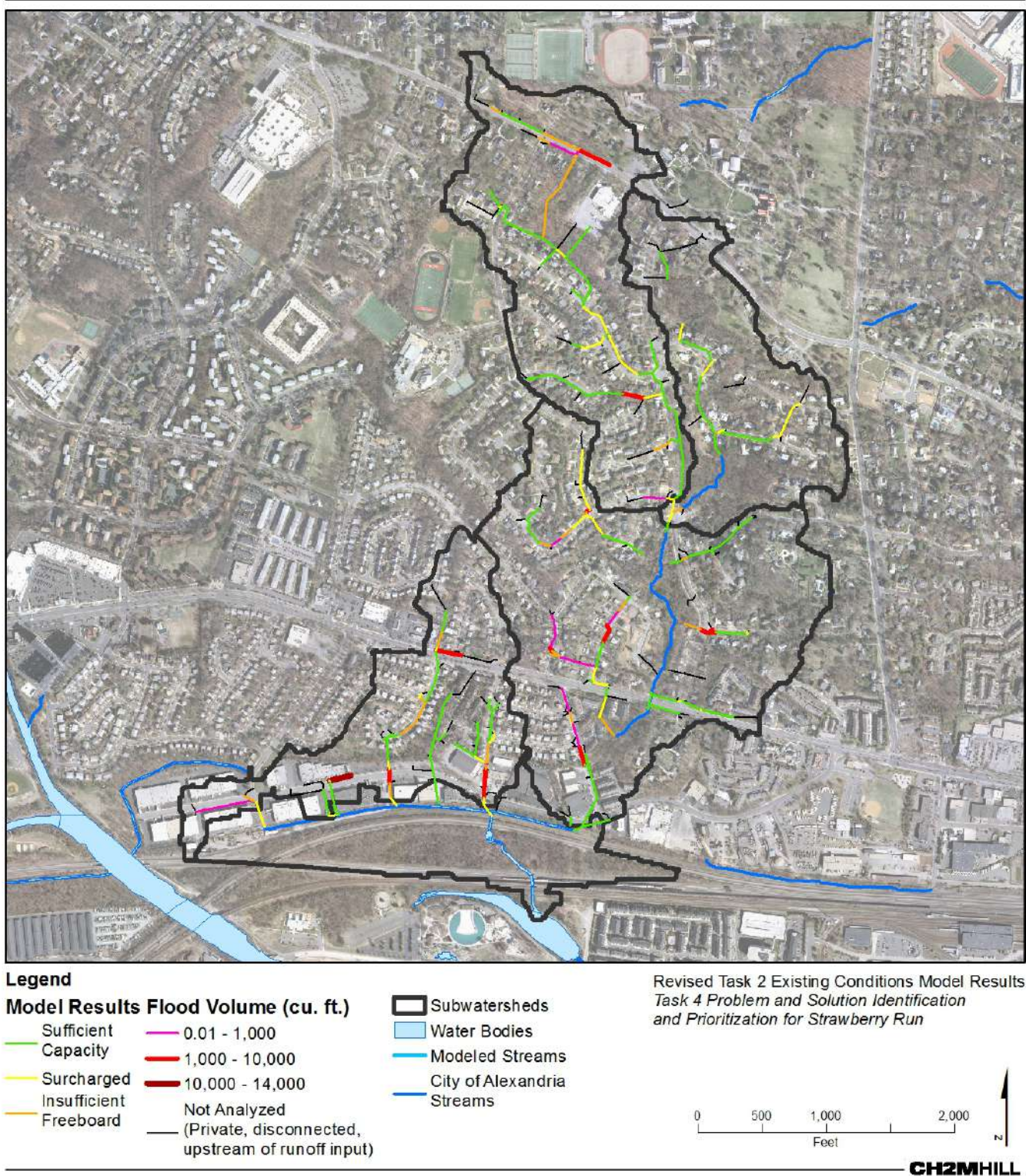
Notes:

Results presented for pipe segments are based on capacity at upstream end of pipe.

<sup>a</sup> Duration of surcharged flow includes time during which conduits have insufficient freeboard or are flooded at upstream end only.

<sup>b</sup> Flooded volume includes volume flooded at upstream end of the conduit.

FIGURE 2-1  
 Revised Task 2 Existing Conditions Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





## 2.2.2 Baseline Improvements and Major Capacity Solutions

In the first watershed analyzed for this study, Hooffs Run, several baseline improvements and major capacity solutions were identified and addressed before evaluating solutions in the rest of the system. The goal of identifying baseline improvements was to remove hydraulic limitations that may have negatively affected the ability to model solutions. A similar evaluation was conducted for Strawberry Run to determine whether baseline improvements and major capacity solutions were needed.

Profiles of the Strawberry Run existing conditions model results were reviewed to identify significant changes in diameter or slope over relatively short distances where there was also a sudden increase in the hydraulic grade line. In addition to reviewing the profiles, the data sources for invert and diameter information were reviewed. There were no locations identified in the Strawberry Run watershed that required baseline improvements. Additionally, no locations were identified within the Strawberry Run watershed where extreme capacity limitations caused long backwater conditions and substantial flooding in the system. Therefore, there was no need to develop solutions for major capacity problems.

## 2.2.3 Alternative Solutions

The purpose of this task was to identify and evaluate corrective measures that could be undertaken to reduce flooding and improve stormwater quality through the use of green infrastructure practices. In addition, there is the potential to achieve other ancillary benefits such as improved aesthetics, urban heat island reduction, and carbon capture through context-sensitive solutions. Potential solutions were developed for each of the following project types or technologies, where applicable:

- Conveyance improvements
- Storage (modeled as underground storage, but could also be implemented as above ground storage or other conventional stormwater management approaches)
- GI

The goal of the conveyance solutions was to evaluate the impact of increased conveyance capacity on flooding and surcharge in the high-priority problem areas. Conveyance improvements were modeled in xpswmm by increasing pipe diameter up to 0.1-foot below ground surface (bgs). The invert elevations and alignment of existing pipes were not altered, so pipe slope did not change from existing conditions. Because the goal of this evaluation was not to design solutions but to evaluate potential strategies and technologies, more detailed design will be required to develop fully implementable projects, including adjusting pipe shapes, providing parallel pipes, and providing for adequate ground cover.

The storage solutions involved evaluating the potential for new detention or retention facilities or offline storage for high-priority problem areas. Because of the dense urban development prevalent in the City, conventional stormwater management (SWM) practices were assumed in the hydraulic model to be limited to offline subsurface storage facilities. Opportunities for subsurface storage were identified in open spaces such as parking lots, green spaces, and grassed medians, with a preference for City-owned properties. Storage was modeled in xpswmm using storage nodes and weirs to model the overflow from a manhole into storage. The maximum storage size was determined by measuring the surface area of the open space available for storage and estimating the storage depth based on the manhole to which the storage system would be dewatered. It was assumed that storage should be a minimum of 3 feet deep and a maximum of 10 feet deep to maintain reasonable construction costs. Additionally, storage was only considered if gravity dewatering to a manhole within 1,000 feet was possible. Storage facilities would not be dewatered until the system had capacity to convey the stored flow. As such—and considering the focus of the modeling was to identify capacity limitations and flooding problems—storage dewatering was not evaluated in this analysis.

GI was evaluated at three different implementation levels: low, medium, and high. In the xpswmm model, GI was modeled by reducing impervious cover in model subcatchments. The low implementation level was modeled as a 10 percent reduction in impervious area, the medium at a 30 percent reduction, and the high at a 50 percent reduction. During development of the modeling approach soil and depression storage parameters were evaluated



for sensitivity in the model. Ideally, these parameters would be adjusted to more accurately represent the physics of GI performance in the field. However, this level of detail in modeling was beyond the scope of this study, and infiltration parameters were not altered when modeling GI.

Table 2-4 describes the modeling approach and basic assumptions for each of the solution technologies. Solutions developed for each high-priority problem area are described in greater detail in Section 4, Solution Identification, of this report.

TABLE 2-4  
Description of Solution Modeling Approaches and Assumptions  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Solution Technology/Strategy	Modeling Approach	Basic Assumptions
Conveyance	Increase Pipe Diameter	Use existing slope and pipe alignment. Increase pipe diameter to a maximum of 0.1-foot bgs. Add barrels as necessary.
Storage	Add storage node with weir to convey flow into storage	Storage depth is between 3 feet and 10 feet bgs. Gravity dewatering is required. A 20-foot-long weir to storage with discharge coefficient of 3 is required. Only surcharged flow will be sent to storage.
Green Infrastructure	Decrease catchment impervious area	Low implementation: 10 percent reduction in impervious area. Medium implementation: 30 percent reduction in impervious area. High implementation: 50 percent reduction in impervious area.

Solution alternatives were modeled in xpswmm. The basis for the solution models was the Task 2 existing conditions model.

Alternative solutions were evaluated in five different models, one for each technology/strategy:

- Conveyance solutions model
- Storage solutions model
- Low GI implementation model
- Medium GI implementation model
- High GI implementation model

This approach has limitations when projects are in close proximity to one another because the hydraulics are inextricably linked. However, because of the number of solutions and technologies being evaluated, evaluating each project independently was not within the scope of the analysis.



## SECTION 3

# Problem Identification

The purpose of the problem identification task was to assign a score to structures in the stormwater drainage network so that high-priority problem areas could be identified. Solution alternatives were developed for high-priority problem areas in the Strawberry Run watershed. Junctions were scored for each of the problem area evaluation criteria. Table 3-1 shows the distribution of scores across the 453 stormwater junctions that were included in Strawberry Run model. The results were generated using the Task 2 existing conditions model (existing IDF and existing boundary conditions) with the model revisions described in Section 2.2 of this TM. A map of the junction scores is provided on Figure 3-1.

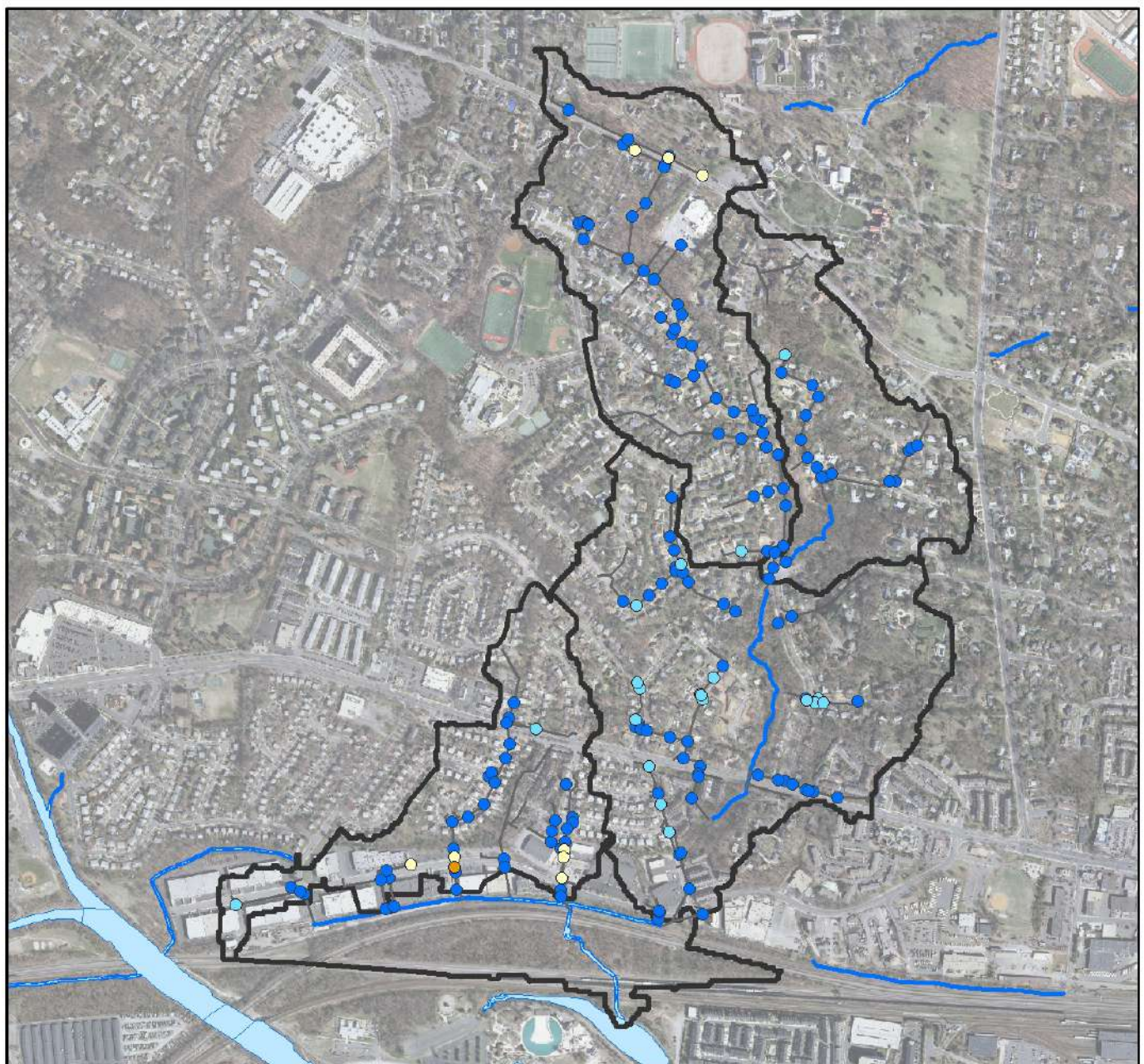
TABLE 3-1  
Strawberry Run Problem ID Scores  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem ID Score	Count of Junctions	% of Total
0	280	61.8
0.1 – 20	142	31.3
20.1 – 30	21	4.6
30.1 – 40	9	2.0
40.1 – 50	1	0.2
>50	0	0.0
<b>Total</b>	<b>453</b>	<b>100</b>

After scoring individual junctions, high-priority problem areas were identified as groupings of hydraulically-connected junctions and pipes in proximity to one another. A total of three high-priority problem areas were identified and are shown on Figure 3-2.



FIGURE 3-1  
 Strawberry Run Problem Identification Score Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*



#### Legend

#### Junction Score

- 0.1 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- > 50

- DGravityMain
- City of Alexandria Streams
- ▭ Subwatersheds
- ▭ Water Bodies

Note: Junction Scores of 0 not shown.

Junction Scores for Revised Task 2 Existing Conditions Model Results

Task 4 Problem and Solution Identification and Prioritization for Strawberry Run

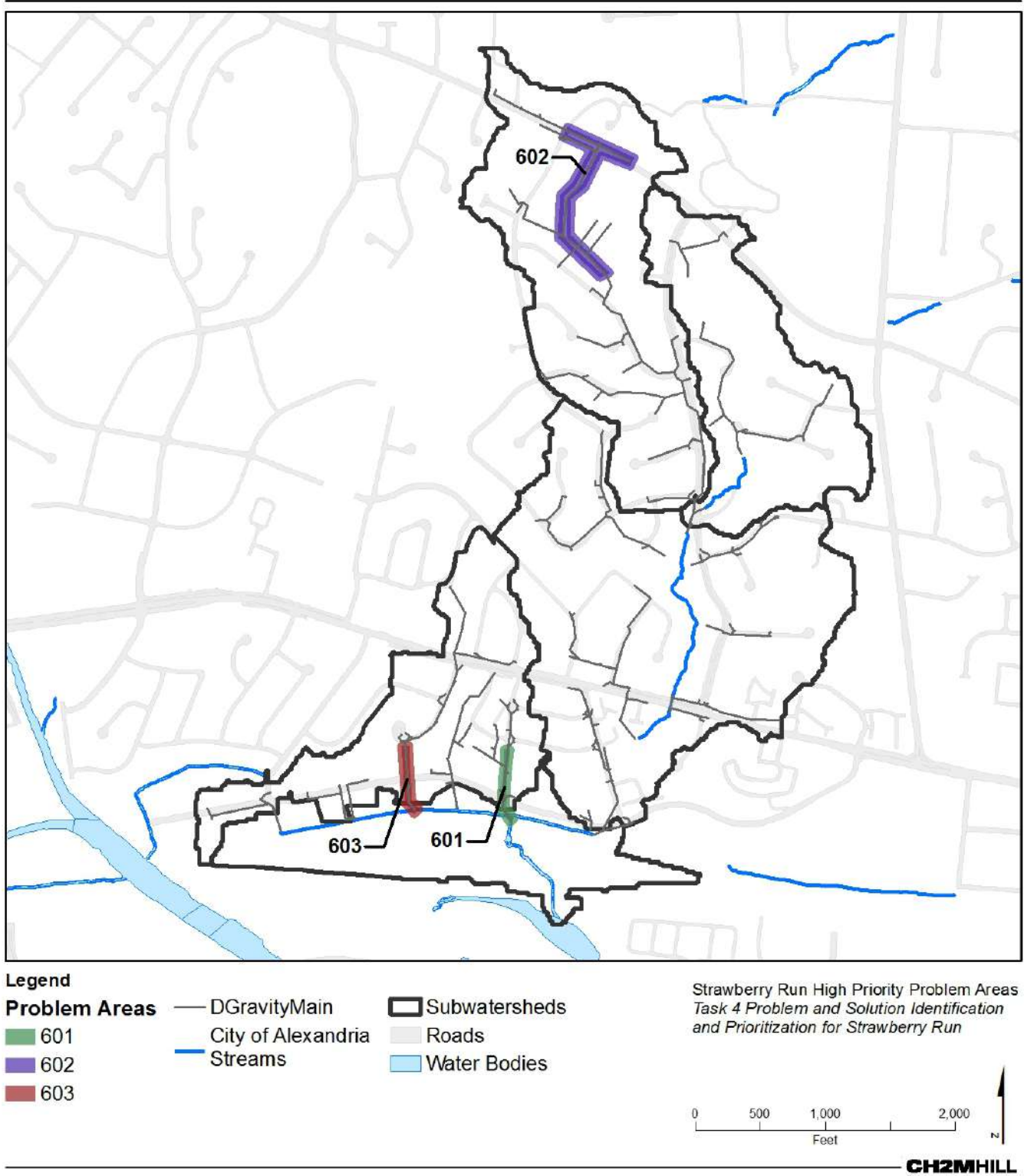
0 500 1,000 2,000  
 Feet

CH2MHILL





FIGURE 3-2  
 Location of Strawberry Run High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





## Solution Identification

A suite of solutions including conveyance, storage, and GI projects, was developed for each problem area. The solution identification process resulted in 15 unique projects for the three high-priority problem areas in the Strawberry Run watershed. Solutions were focused on the high-priority problem areas; therefore, flooding outside those problem areas would not necessarily be addressed by any of the alternatives. For example, flooding west of Problem Area 603, shown in Figure 4-1, is not near critical infrastructure or roadways and has not been identified as a problem by City staff or local residents, so this area did not score high enough in the problem identification step to be classified as a high-priority problem area.

### 4.1 Conveyance Solutions

A conveyance solution was developed for each of the high-priority problem areas. The goal of the conveyance solutions was to remove hydraulic limitations in the drainage network by increasing the capacity of the pipes in high-priority problem areas. Because this was a high-level conceptual exercise rather than a design exercise, the pipe alignment and roughness were left unchanged and capacity was increased solely by increasing the pipe size. In most cases, pipe shape was not altered except where sufficient capacity could not be achieved because of limited cover or where the existing pipe was a special shape, such as horizontal elliptical pipes. Where there was limited cover, circular pipes were changed to box culverts so that capacity could be increased without daylighting. Special pipe shapes were converted to equivalent-diameter circular pipes to simplify the model and calculations.

The conveyance capacity required was estimated using xpswmm. A hydraulic model was used to approximate the unconstrained peak flow in each pipe segment by upsizing pipes to 0.1-inch bgs to maximize diameter without daylighting the pipe, and by increasing the number of barrels by a factor of 2 across the board. The resulting unconstrained peak flow and Manning's equation were used to back-calculate the diameter required for the pipe to flow less than 80 percent full.

In the high-priority problem areas, the required diameter was compared to the existing diameter. Pipes that were smaller than the required pipe size calculated using the unconstrained peak flow were upsized and included in the conveyance project. Pipes that had sufficient capacity under existing conditions were left unchanged. Pipe size was not optimized during this exercise, and runs of pipes were not consistently sized. A summary of the length of pipe and range of pipe sizes included in each conveyance solution is included in Table 4-1. A table documenting the existing and proposed diameter of each pipe segment is provided in Appendix A.

TABLE 4-1  
Summary of Conveyance Projects  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Project ID	Replacement Pipe Size Range and Project Description	Length (LF)
601	CONV-601	18-30 inch Replacement Sewer Pipe Relief	586
602	CONV-602	18-54 inch Replacement Sewer Pipe Relief	1,862
603	CONV-603	36-72 inch Replacement Sewer Pipe Relief	513

A map of the results of the existing conditions model results is provided on Figure 4-1 for reference, and a map of the conveyance solutions model results is provided on Figure 4-2. A summary of the modeling results is provided in Table 4-2.





FIGURE 4-1  
Existing Conditions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

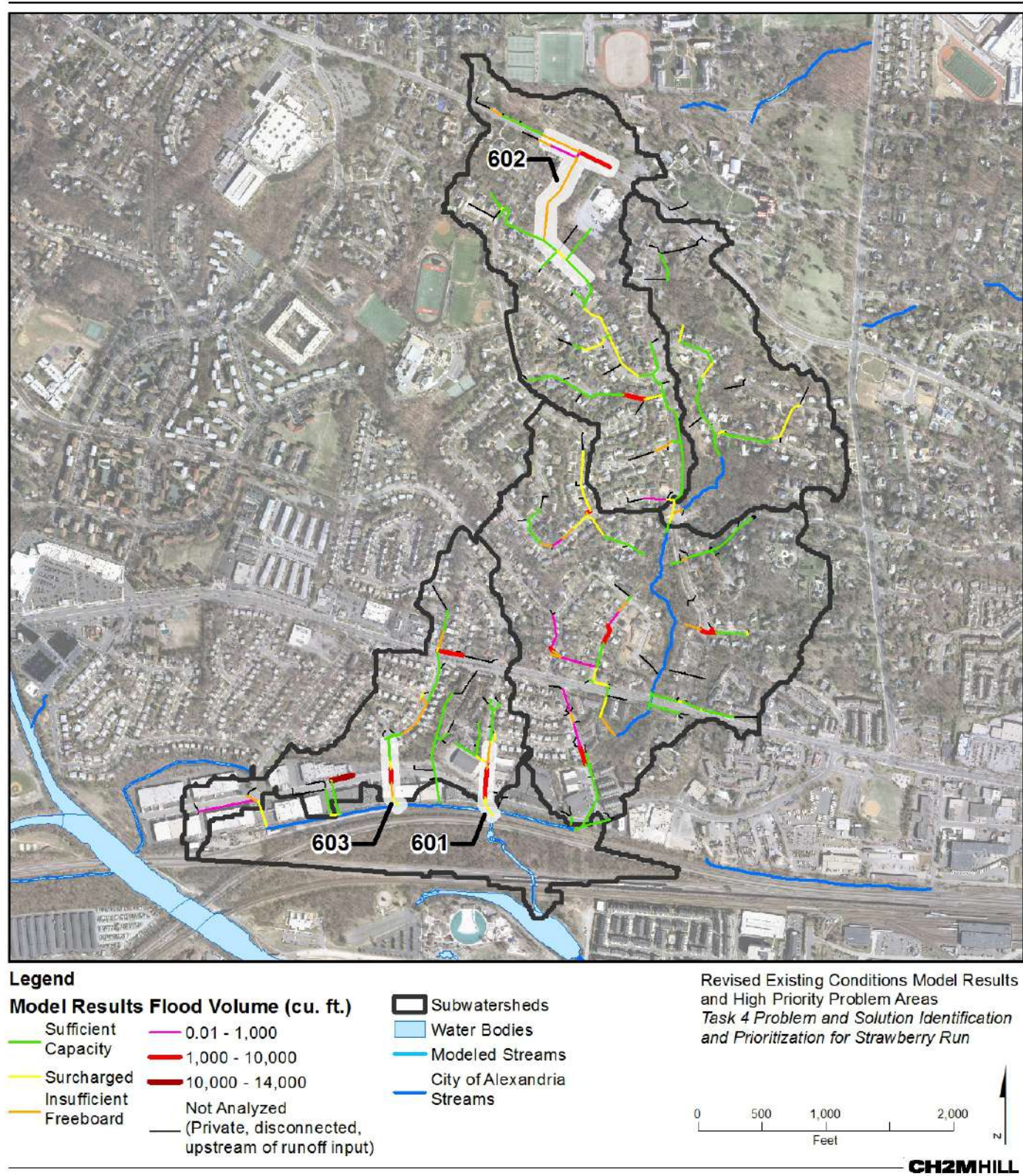
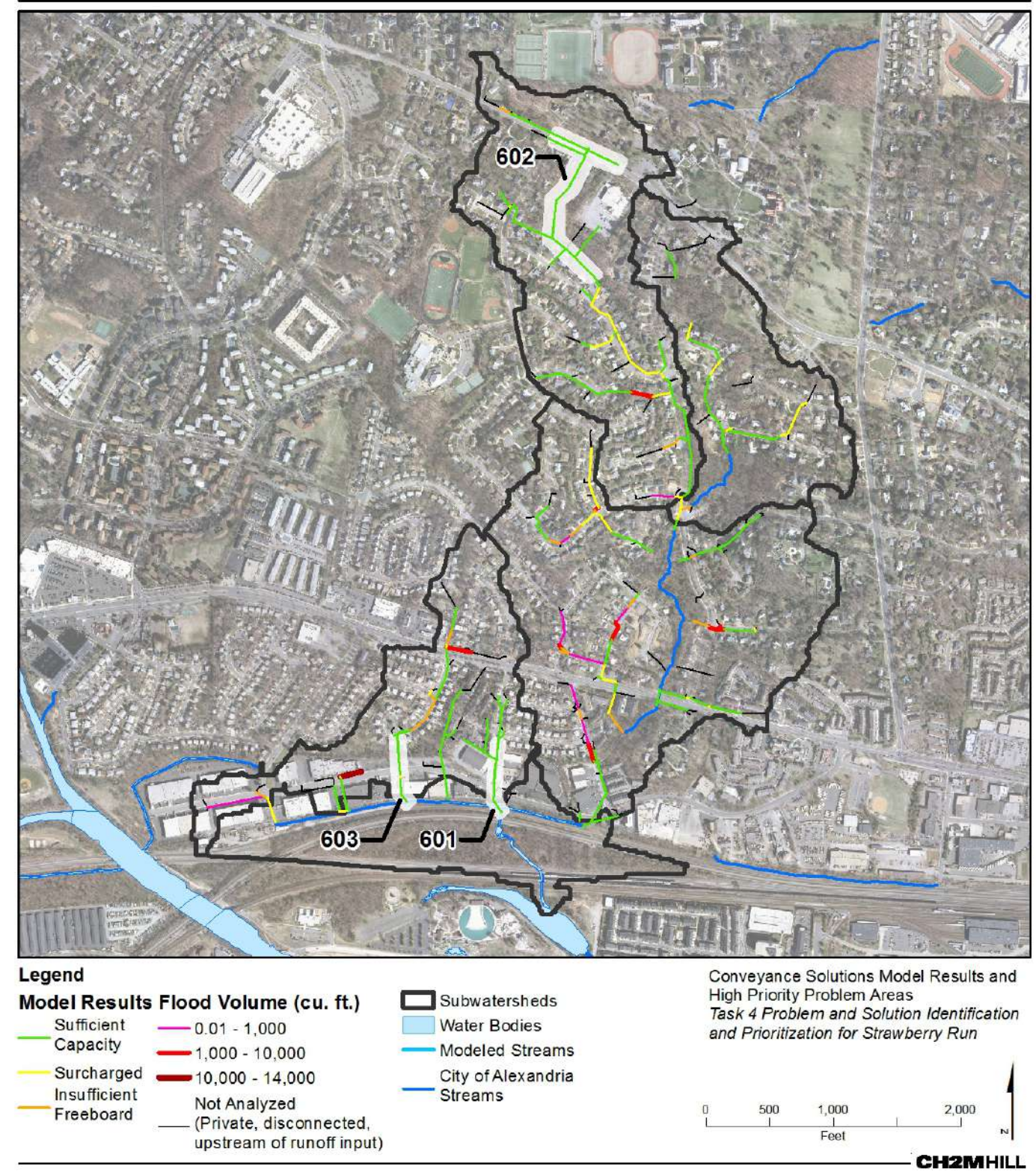


FIGURE 4-2  
Conveyance Solutions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*







The conveyance solutions lessened and/or resolved most of the localized problems within the high-priority problem areas. In Strawberry Run there is a limited amount of collection system downstream of the high-priority problem areas, which limits downstream impacts to the closed conduit collection system; however, the increased peak flow could have detrimental effects on the stream channel into which the storm system discharges, but impacts to the stream were not analyzed in this study. Table 4-2 summarizes the model results for the existing conditions and the conveyance solutions models. Comparing the two results shows that overall flooding is eliminated in about 3 percent of the system by length. The total volume flooded is reduced by about 15 percent, and the duration of surcharge and flooding are reduced by 17 and 15 percent, respectively.

TABLE 4-2  
Summary of Existing Conditions and Conveyance Solutions Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Existing Conditions Results				Conveyance Solutions Results			
	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>
Sufficient Capacity	13,802	54	-	-	16,424	65	-	-
Surcharged <sup>a</sup>	4,308	17	35	-	4,107	16	29	-
Insufficient Freeboard	3,392	13	-	-	1,927	8	-	-
Flooded	3,877	15	13	67,657	2,921	12	11	57,606

Notes:

Results presented for pipe segments are based on capacity at upstream end of pipe.

<sup>a</sup> Duration of surcharged flow includes time during which conduits are surcharged, have insufficient freeboard, or are flooded at upstream end only.

<sup>b</sup> Flooded volume includes volume flooded at upstream end of the conduit.

Flooding outside of the high-priority problem areas was not addressed by the proposed solutions; therefore, a summary of the modeling results within the high-priority problem areas is provided in Table 4-3. Flooding was eliminated within all high-priority problem areas. The disadvantage of conveyance solutions is that, although increasing pipe capacity reduces flooding in the problem area, it increases peak flows, which may create or increase peaks in the stream channel or flooding downstream. Peak flow was increased for all three high-priority problem areas, although this increase was much higher in some problem areas, ranging from an 8 percent increase in Problem Area 602 to a 35 percent increase in Problem Area 601.

TABLE 4-3  
Conveyance Solutions Model Results by Problem Area  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Flood Volume (MG)			Peak Flow at Downstream End of Problem Area (cfs)		
	Existing Conditions Model Results	Conveyance Solution Model Results	Percent Reduction	Existing Conditions Model Results	Conveyance Solution Model Results	Percent Increase
601	0.04	-	100	31.8	42.8	35
602	0.02	-	100	120.7	130.3	8
603	0.02	-	100	79.6	88.0	10
Average			100	18		

Notes:

MG = million gallons

cfs = cubic feet per second

Taking the approach of sizing the conveyance projects based on the unconstrained peak flow allowed all conveyance projects to be run in a single iteration. Because stormwater gravity main diameters were increased to

convey the largest potential peak flow, the impact of increasing capacity upstream was incorporated into the sizing of any downstream conveyance solutions. However, evaluating all of the conveyance projects in a single model run has limitations. Because the problem areas are interconnected, modeling all solutions in a single run does not allow each solution to be viewed independently. In Strawberry Run, the three problem areas are distributed throughout the watershed and therefore have a limited impact on one another.

## 4.2 Storage Solutions

Conventional SWM solutions considered in this study include detention facilities and ordinance changes. Because of the challenges of translating ordinance changes into hydrologic and hydraulic parameters, only storage solutions were modeled in xpswmm. Ordinance changes were reviewed during the Hooffs Run Task solutions analysis and are summarized in *Task 4: Problem and Solution Identification and Prioritization for Hooffs Run, Alexandria, Virginia* (CH2M HILL, 2016b).

The goal of storage solutions was to add storage to the stormwater drainage network to decrease peak flow and volume during the modeled rainfall event. Because of the urban nature of the study area, it was assumed that to provide a sufficient storage volume, detention facilities would have to be below-grade vaults. Several constraints guided the siting of potential storage solutions, including:

- Depth of storage facility should not exceed 10 feet to minimize excavation costs.
- Storage will be dewatered by gravity to a manhole less than 1,000 feet downstream to eliminate pumping costs.
- Minimum storage depth should be 3 feet, measured from the storage inlet to the storage outlet.
- Only surcharged flow will be sent to storage.

The first step in developing storage solutions was to identify open space that may be available for subsurface storage vaults with preference for City-owned property. This primarily included parking lots, green space (such as parks, school yards, playing fields, church yards), and grassed medians or boulevards. These opportunities were identified using aerial imagery and were deemed feasible using drainage network data (gravity main locations and inverts) and topographic data. Storage areas meeting the constraints described above were identified for all three of the high-priority problem areas. A map of these locations is provided on Figure 4-3, and Table 4-4 summarizes the storage depth, area, and volume. More details of the storage solution locations are provided in Appendix B.

TABLE 4-4  
Storage Solutions Summary  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Storage ID	Max Depth (ft)	Total Storage Area Available (ft <sup>2</sup> )	Total Volume Available (ft <sup>3</sup> )	Total Volume Required (ft <sup>3</sup> )
601	601	10.0	2,108	21,080	8,901
602	602	5.8	4,031	23,501	1,310
603	603	4.2	5,798	24,352	23,974

A map of the results of the storage solution model run is provided on Figure 4-4, and a summary of the results is provided in Table 4-5.

FIGURE 4-3  
 Storage Solution Locations and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

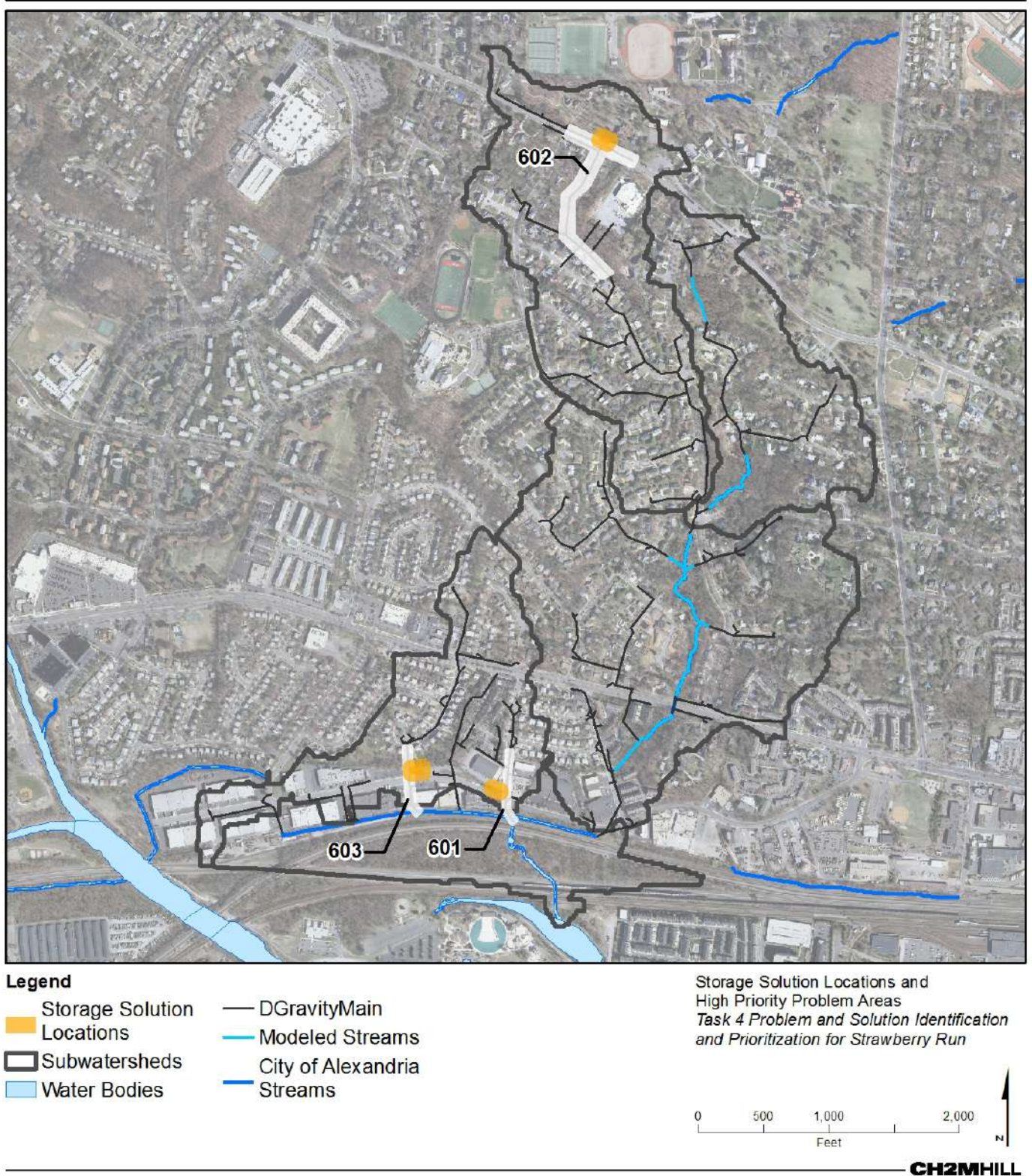






FIGURE 4-4  
 Storage Solutions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

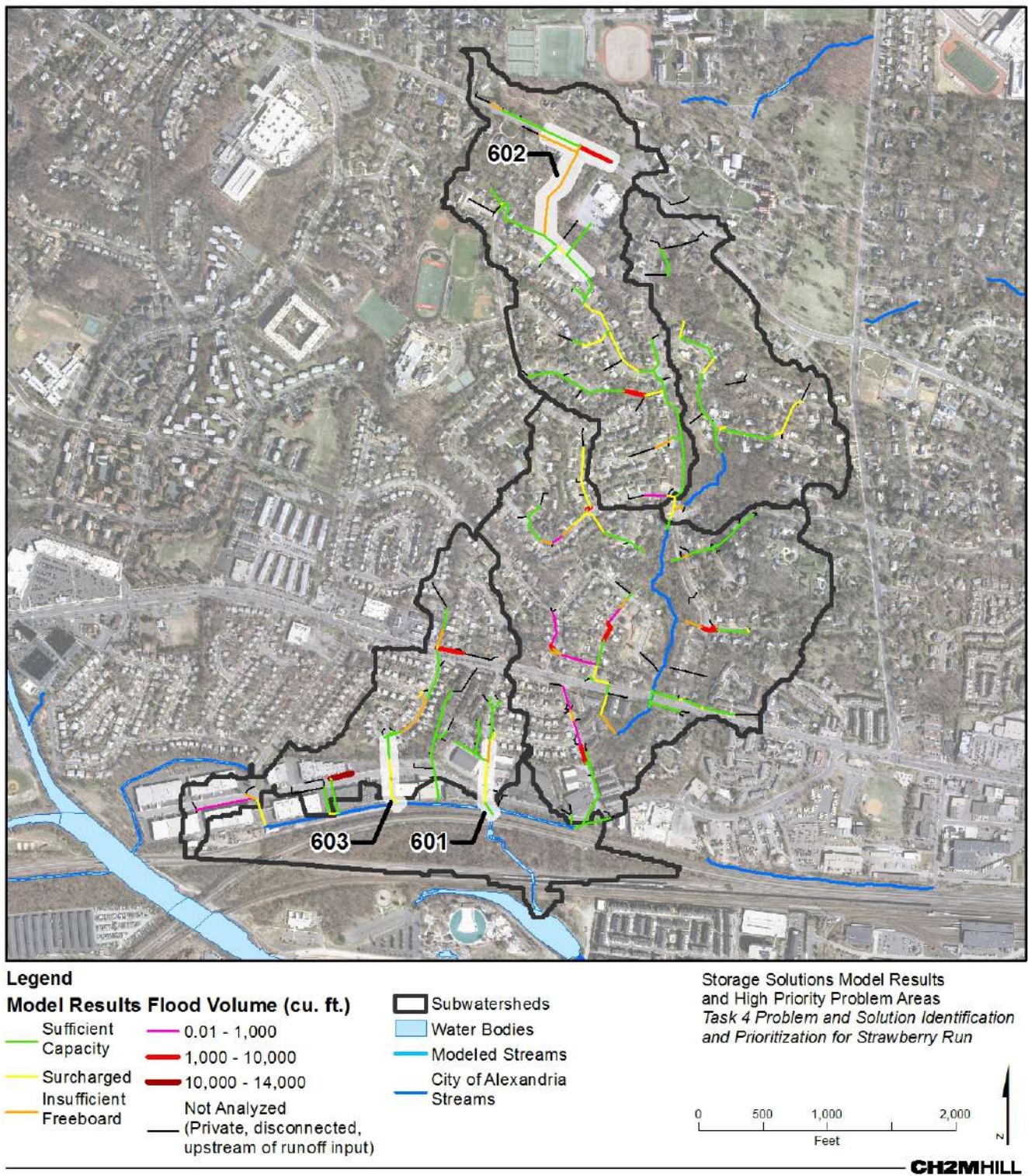






TABLE 4-5  
Summary of Existing Conditions and Storage Solutions Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Existing Conditions Results				Storage Solutions Results			
	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>
Sufficient Capacity	13,802	54	-	-	14,343	57	-	-
Surcharged <sup>a</sup>	4,308	17	35	-	4,560	18	33	-
Insufficient Freeboard	3,392	13	-	-	3,287	13	-	-
Flooded	3,877	15	13	67,657	3,188	13	11	59,021

Notes:

Results presented for pipe segments are based on capacity at upstream end of pipe.

<sup>a</sup> Duration of surcharged flow includes time during which conduits are surcharged, have insufficient freeboard, or are flooded at upstream end only.

<sup>b</sup> Flooded volume includes volume flooded at upstream end of the conduit.

Overall, the storage solutions decrease the total volume of flooding in the watershed by almost 13 percent, and the duration of flooding is also decreased by 15 percent. Flooding is eliminated in about 2 percent of the system by length, which also produces a slight increase in percentage of the system with sufficient capacity. The total portion of the system surcharged or with insufficient freeboard and the duration of surcharged were not significantly impacted.

Flooding outside of the high-priority problem areas was not addressed by the proposed solutions; therefore, Table 4-6 summarizes the modeling results within the high-priority problem areas. On average, the flood volume and peak flow reductions within the high-priority problem areas are 78 and 15 percent, respectively.

TABLE 4-6  
Storage Solutions Model Results by Problem Area  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Flood Volume (MG)			Peak Flow at Downstream End of Problem Area (cfs)		
	Existing Conditions Model Results	Storage Solution Model Results	Percent Reduction	Existing Conditions Model Results	Storage Solution Model Results	Percent Reduction
601	5,089	-	100	31.8	27.3	14
602	2,155	1,415	34	120.7	120.6	0
603	2,697	-	100	79.6	55.4	30
Average			78	15		

Evaluating all of the storage solutions in a single model is not limited by increased downstream impacts as the conveyance solutions are. Instead, because of the increased storage capacity at upstream problem areas, the full peak flow may not reach downstream problem areas. In this case, the performance of a problem area may appear to be more favorable than if each problem area were modeled separately. However, since the high-priority problem areas are distributed throughout the watershed, storage added in one problem area will have a limited impact on another in Strawberry Run.

## 4.3 Green Infrastructure Solutions

The goal of GI solutions is to reduce the peak runoff rate and runoff volume directed to the storm drainage system by converting impervious surfaces to pervious surfaces. This is accomplished in the field by redirecting runoff from impervious surfaces to GI facilities that detain and infiltrate runoff during rainfall events. Three levels of GI—low, medium, and high—were evaluated in this analysis. In the model, GI was evaluated by reducing the impervious cover in model subcatchments by 10 percent, 30 percent, and 50 percent to represent the low, medium, and high levels of implementation, respectively.

Several GI technologies were considered feasible within the City, including:

- **Bioretention/ Planters** – Planted depression or constructed box with vegetation that typically receives runoff from roadways or rooftops; includes vegetation and soil media over an underdrain and filtration fabric. The City does not typically encourage infiltration; therefore, rain gardens, which typically do not have an underdrain, are not encouraged.
- **Cisterns** – A tank for storing water, typically connected to a roof drain that can be either above or below ground. Water from a cistern is typically reused or slowly infiltrated into the soil rather than discharged to a storm sewer.
- **Green/Blue Roofs** – A roof of a building that is partially or completely covered with vegetation and a growing medium, planted over a waterproofing membrane (green roof) or a roof that is capable of storing and then slowly releasing rainwater (blue roof).
- **Porous Pavement** – Paving surfaces designed to allow stormwater infiltration; may or may not include underground storage component.
- **Surface Storage** – Retrofit of inlets and catch basins to include flow regulators on streets with a standard curb and gutter system so that stormwater can be stored within the roadway and slowly released back into the storm sewer system.
- **Amended Soils** – Altering soils to improve water retention, permeability, infiltration, drainage, aeration, and/or structure.

These technologies were grouped into GI programs based on the land uses where they could be applied. A program combines a set of technologies into an implementation strategy for different types of sites and land use categories. Programs being considered are described as follows:

- **Green Streets/Alleys** – Includes bioretention/planters and porous pavement combined along the public right-of-way (ROW) between buildings and roadways; can include parking lane and curb cuts.
- **Green Roofs** – Includes green/blue roofs, sometimes in combination with cisterns.
- **Green Schools** – Use of school properties to implement one-to-many GI management strategies, including bioretention/planters, cisterns, green/blue roofs, and porous pavement.
- **Green Parking** – Bioretention/planters and porous pavement in parking lots.
- **Green Buildings** – Use of bioretention/planters, cisterns, and/or downspout disconnection on public or private buildings.
- **Blue Streets** – Short-term surface storage on streets with relatively flat slopes and standard curb and gutter systems.
- **Open Spaces** – Use of open spaces to store and/or infiltrate stormwater using a combination of detention, amended soils, bioretention/planters, and/or porous pavement; may also include stream daylighting where appropriate.

Six GI concepts were developed for the Strawberry Run watershed. These concepts, which are described in greater detail in Appendix C, demonstrate the applicability of GI technologies in the Strawberry Run watershed.

A drainage area for each high-priority area was identified using the model's hydrologic subcatchments. Table 4-7 summarizes the drainage area, existing impervious area, and impervious area for each level of GI implementation. A map of these drainage areas and problem area locations is provided on Figure 4-5.

TABLE 4-7  
Green Infrastructure Solutions Summary  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Drainage Area (acres)	Existing Impervious Area (acres)	Green Infrastructure Solution Impervious Area (acres)		
			Low Implementation	Medium Implementation	High Implementation
601	8.7	5.5	5.0	3.9	2.8
602	41.0	12.0	10.8	8.4	6.0
603	23.4	9.5	8.6	6.7	4.7

Maps of the results of the low, medium, and high GI solutions are provided on Figures 4-6 through 4-8, and a summary of the model results is provided in Table 4-8.



FIGURE 4-5  
 Green Infrastructure Drainage Areas and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

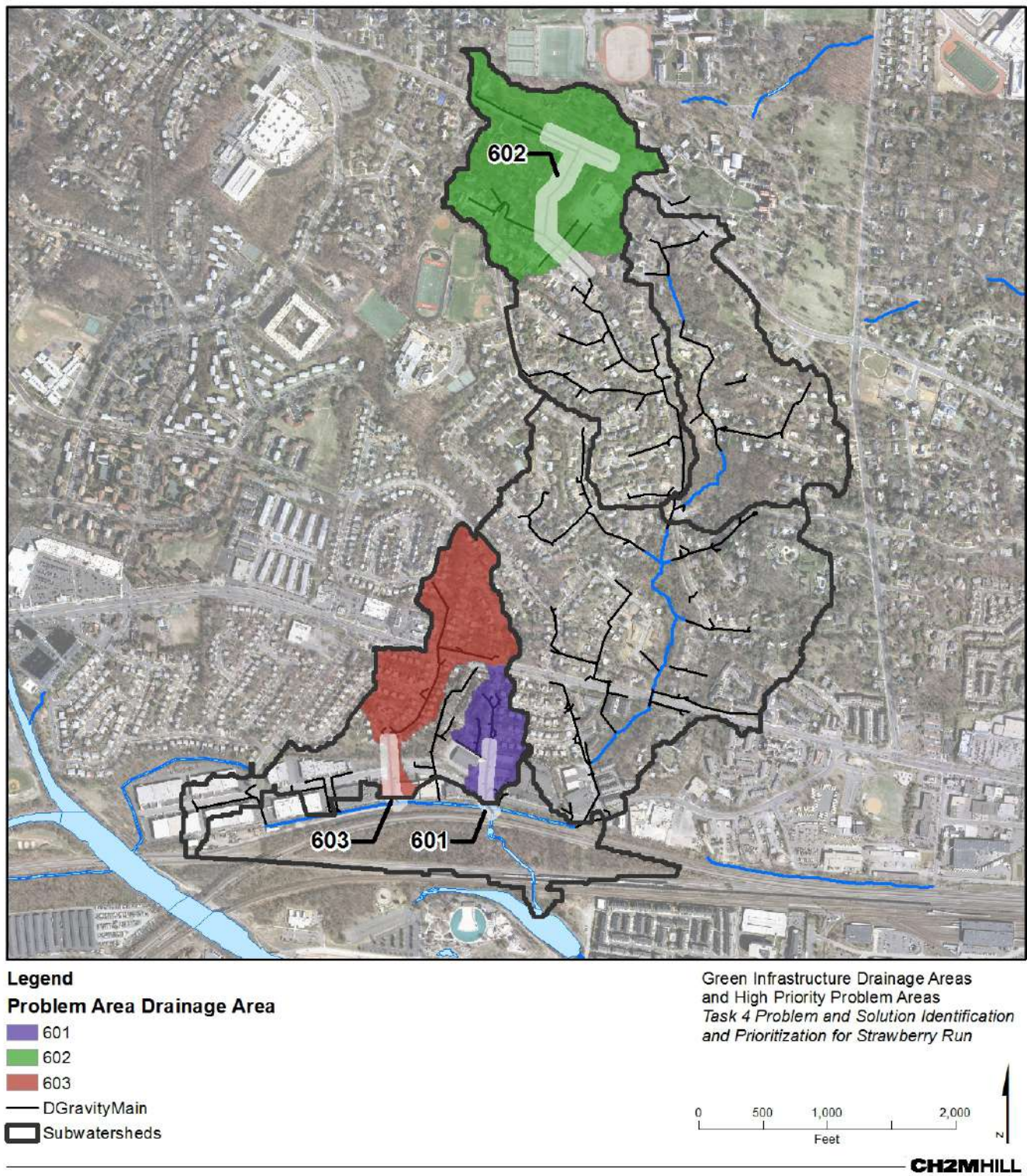






FIGURE 4-6

Low-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

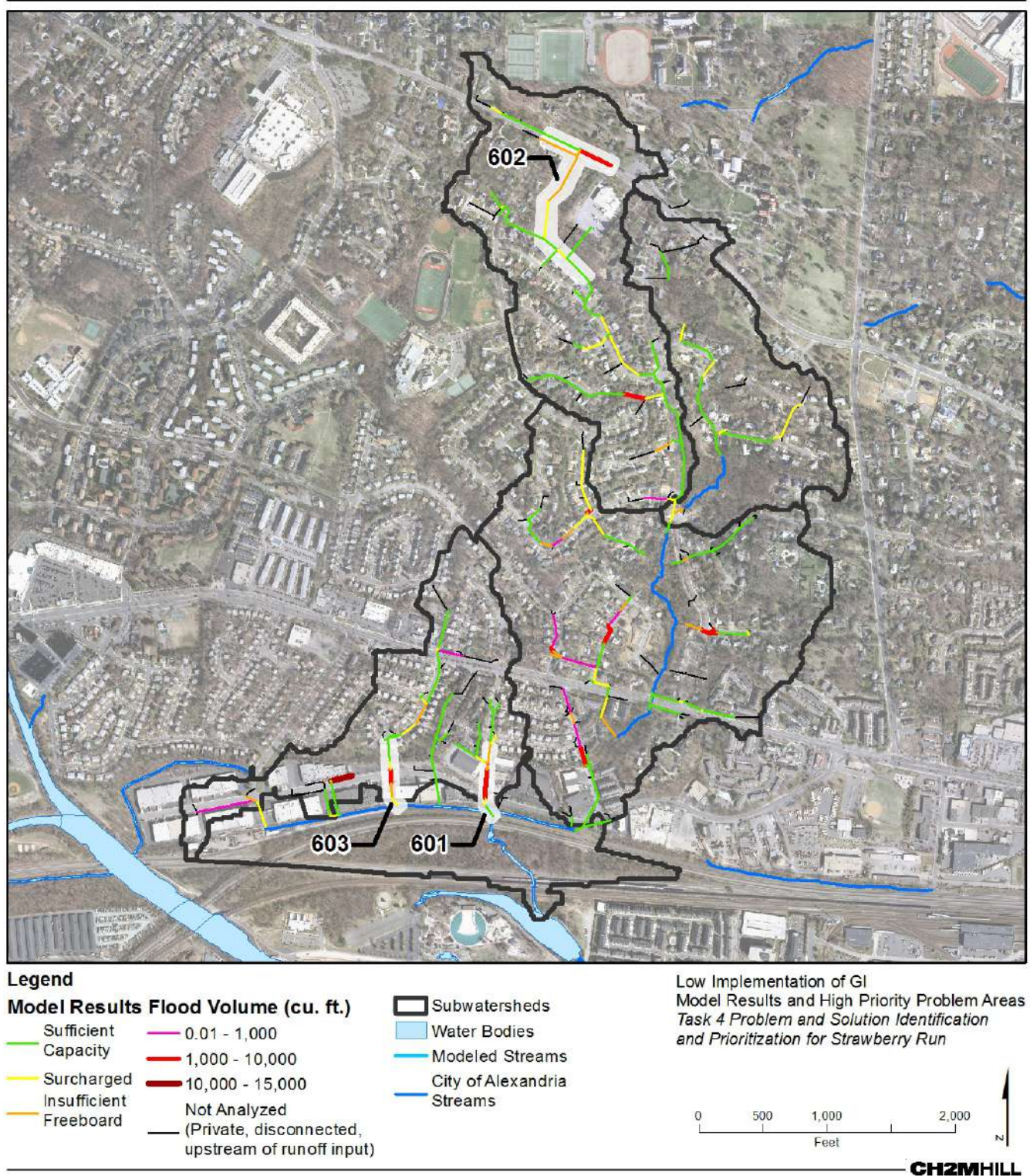






FIGURE 4-7

Medium-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

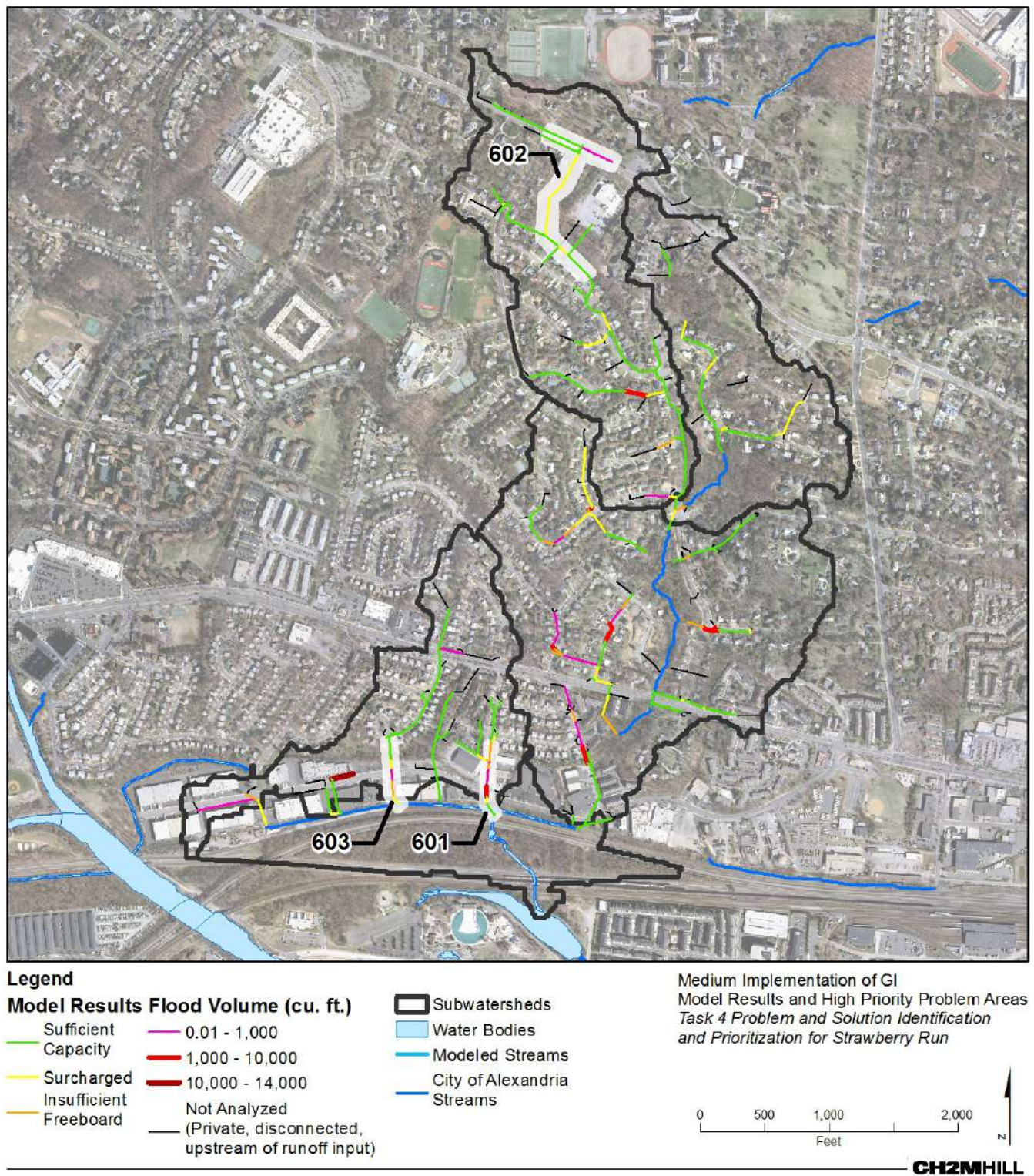






FIGURE 4-8

High-implementation Green Infrastructure Solutions Model Results and High-priority Problem Areas  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

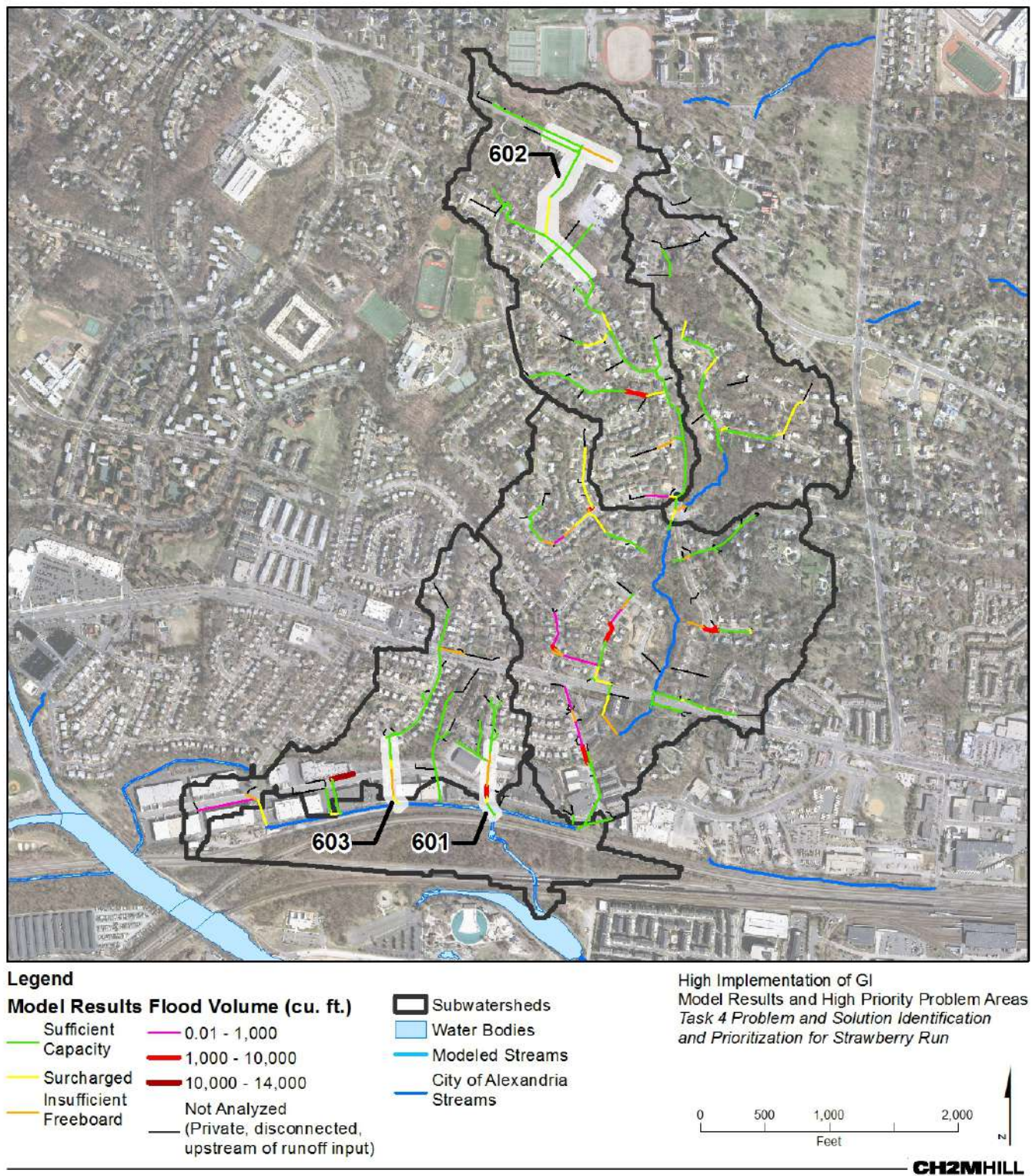




TABLE 4-8

Summary of Existing Conditions and Green Infrastructure Implementation Model Results

*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Existing Conditions Results				Low Green Infrastructure Implementation Results				Medium Green Infrastructure Implementation Results				High Green Infrastructure Implementation Results			
	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft3) <sup>b</sup>
Sufficient Capacity	13,802	54	-	-	14,540	57	-	-	16,037	63	-	-	16,807	66	-	-
Surcharged <sup>a</sup>	4,308	17	35	-	4,593	18	34	-	4,134	16	32	-	3,459	14	30	-
Insufficient Freeboard	3,392	13	-	-	2,702	11	-	-	1,664	7	-	-	2,316	9	-	-
Flooded	3,877	15	13	67,657	3,544	14	13	64,810	3,544	14	12	59,924	2,797	11	11	57,659

## Notes:

Results presented for pipe segments are based on capacity at upstream end of pipe.

<sup>a</sup> Duration of surcharged flow includes time during which conduits are surcharged, have insufficient freeboard, or are flooded at upstream end only.<sup>b</sup> Flooded volume includes volume flooded at upstream end of the conduit.

Overall, model results showed that GI capable of reducing flood volumes and durations in the Strawberry Run storm sewer system. However, because the GI was only modeled in the problem area drainage areas, which only covers about 20 percent of the Strawberry Run watershed, GI does not have a significant impact on the watershed as a whole. The Low Implementation level, which corresponds to a 10 percent reduction in impervious area, had limited benefit when compared to existing conditions. The Medium and High Implementation levels, which reduced imperviousness by 30 percent and 50 percent respectively, performed better. Medium implementation of GI reduced flooding volume by about 10 percent and duration of flooding by about 20 percent. High implementation of GI reduced total flood volume by about 15 percent and duration of flooding by about 30 percent.

Flooding outside of the high-priority problem areas was not addressed by the proposed solutions; therefore, results within each high-priority problem area are shown in Tables 4-9 and 4-10. On average, the flood volume was reduced by 27 percent in high-priority problem areas by the Low GI implementation, 74 percent by the Medium GI implementation, and 92 percent by the High GI implementation solution. Peak flow results were less dramatic though still significant at the Medium and High GI implementation levels, with peak flows reduced by about 6 and 10 percent for Medium and High GI implementation respectively.

TABLE 4-9  
Green Infrastructure Solutions Flood Volume Model Results by Problem Area  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Existing Conditions Flood Volume (MG)	Low GI Implementation		Medium GI Implementation		High GI Implementation	
		Solution Flood Volume (MG)	Percent Reduction	Solution Flood Volume (MG)	Percent Reduction	Solution Flood Volume (MG)	Percent Reduction
601	5,089	4,285	16	2,718	47	1,179	77
602	2,155	1,091	49	85	96	-	100
603	2,697	2,246	17	570	79	-	100
		<b>Average</b>	<b>27</b>		<b>74</b>		<b>92</b>

TABLE 4-10  
Green Infrastructure Solutions Peak Flow Model Results by Problem Area  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Existing Conditions Peak Flow (cfs)	Low GI Implementation		Medium GI Implementation		High GI Implementation	
		Solution Peak Flow (cfs)	Percent Reduction	Solution Peak Flow (cfs)	Percent Reduction	Solution Peak Flow (cfs)	Percent Reduction
601	31.8	31.8	0	31.4	1	30.8	3
602	120.7	117.3	7	109.4	9	98.5	18
603	79.6	74.0	3	74.0	7	74.0	7
		<b>Average</b>	<b>3</b>		<b>6</b>		<b>10</b>

# Alternatives Analysis and Prioritization

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The goal of alternatives analysis and prioritization was to evaluate the cost and performance of the various solution approaches/technologies and develop watershed-wide alternatives aimed at resolving capacity-related problems in the Strawberry Run watershed. The solution identification process resulted in 15 unique projects for the three high-priority problem areas in the Strawberry Run watershed. The alternatives analysis and prioritization was performed after completing the solution modeling for the high-priority problem areas. The following section describes the results of the alternatives analysis and prioritization.

## 5.1 Problem Area Benefit Analysis

The 15 solutions for the three high-priority problem areas were scored for the each of the following solution evaluation criteria:

- Urban drainage/flooding
- Environmental compliance
- EcoCity goals/sustainability
- Social benefits
- Integrated asset management
- City-wide maintenance implications
- Constructability
- Public acceptability

After completing preliminary scoring of all projects, City staff reviewed prioritization results to be sure the objectives of the analysis were being met. This review resulted in a minimum flood reduction threshold of 22 percent for all projects. If projects did not meet this minimum threshold, they were not included in the prioritization, although the scoring and costing data were maintained for documentation. Because Low GI was not particularly effective at reducing flooding in the Strawberry Run watershed, nearly all of the Low GI solutions were eliminated by the minimum flood reduction threshold. Of the 15 solutions, 2 Low GI solutions did not meet the minimum flood reduction threshold, leaving 13 projects.

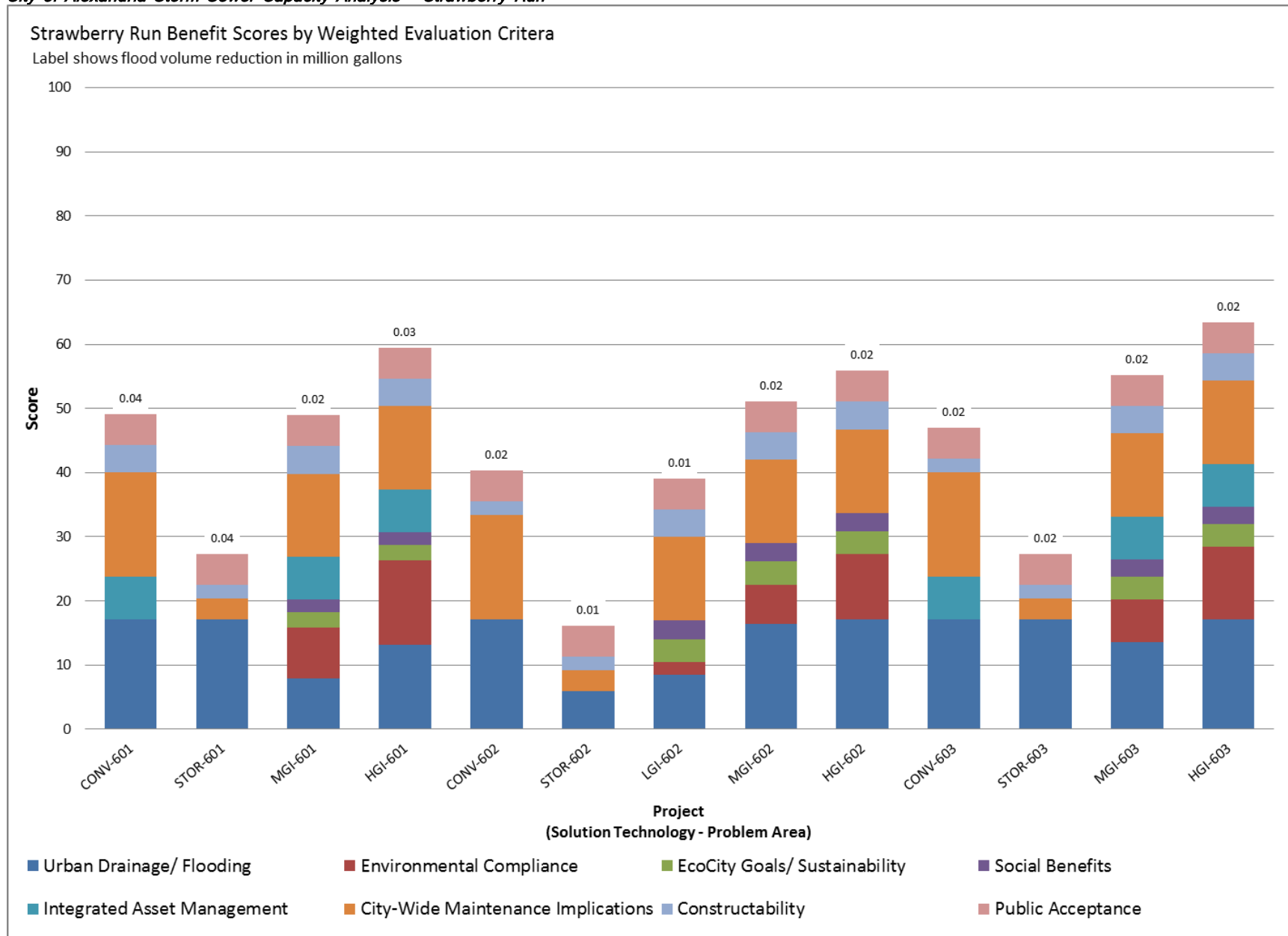
Figure 5-1 is a bar chart of the total benefit scores for each of the 13 projects that meet the minimum flood reduction threshold. The horizontal axis has the project name, which is a combination of the problem area number and the technology/solution approach type. For example, CONV-601 is the conveyance solution for Problem Area 601; STOR-601 is the storage solution; and LGI-601, MGI-601, and HGI-601 are the low, medium, and high GI implementations, respectively. The charts show all solutions included in the prioritization (that is, all solutions providing at least 22 percent reduction in flooding) by problem area in ascending order from left to right.

A full table of the scoring and alternatives analysis results is included in Appendix D.





FIGURE 5-1  
 Total Benefit Score Chart for High-priority Problem Areas 601 through 603  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





## 5.2 Problem Area Solution Costs

Planning-level capital costs, which include construction as well as engineering and design and contingency, were developed for each of the 15 solutions. The basis of the cost information for each technology is provided in Appendix E. The basic unit costs used for costing the various projects were the same across all City infrastructure projects. Three levels of GI implementation were evaluated for this project:

- High Implementation – Manage 50 percent of total impervious area in the watershed
- Medium Implementation – Manage 30 percent of total impervious area in the watershed
- Low Implementation – Manage 10 percent of total impervious area in the watershed

The unit cost of implementing GI at the various implementation levels is driven by the availability of GI opportunity areas. Because the GI opportunity areas varied across watersheds, the cost of implementation of the various levels of GI also varies across watersheds. Table 5-1 provides the construction cost assumptions for low, medium, and high implementation levels of GI in the Strawberry Run watershed based on implementing GI across the whole watershed.

TABLE 5-1  
Strawberry Run Green Infrastructure Construction Costs  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Green Infrastructure Level	Area Managed		Cost per Acre Managed	Construction Cost
	%	Acres		
Low Green Infrastructure	10	11.2	\$48,714	\$543,643
Medium Green Infrastructure	30	33.5	\$85,901	\$2,875,970
High Green Infrastructure	50	55.8	\$140,095	\$7,817,299

Table 5-2 provides the capital cost, in millions of dollars, for all 15 solutions. Projects that do not meet the minimum threshold for flood reduction are shown in ***bold italics***.

TABLE 5-2  
Capital Costs for High-priority Problem Area Solutions  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Conveyance	Storage	Low Green Infrastructure	Medium Green Infrastructure	High Green Infrastructure
601	\$0.38	\$0.16	<b><i>\$0.04</i></b>	\$0.20	\$0.54
602	\$0.73	\$0.05	\$0.08	\$0.43	\$1.18
603	\$0.35	\$0.38	<b><i>\$0.06</i></b>	\$0.34	\$0.93
<b>Total</b>	<b>\$1.45</b>	<b>\$0.58</b>	<b>\$0.18</b>	<b>\$0.98</b>	<b>\$2.66</b>

Note:

Costs shown in ***bold italics*** are for projects that do not meet the 22 percent minimum flood reduction threshold set by the City. Costs are in millions of dollars.

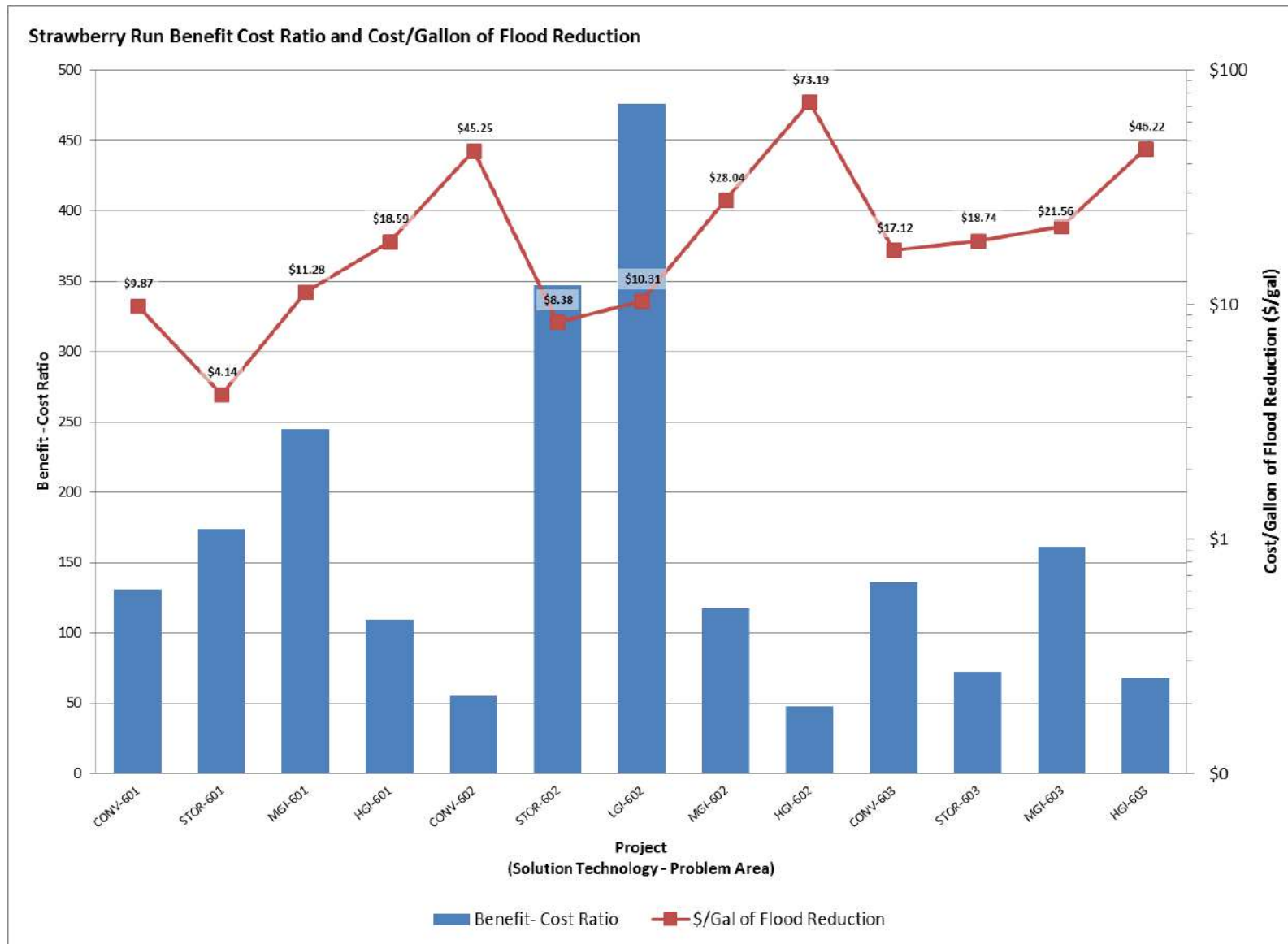
## 5.3 Problem Area Benefit/Cost Results

The benefit/cost score is the ratio of the total benefit divided by the total capital cost in millions of dollars. This metric indicates the cost efficiency of a project and can help direct resources to the projects that will provide the greatest benefit for the lowest cost. Cost benefit results are presented on Figure 5-2. The chart shows only those projects meeting the 22 percent minimum flood reduction threshold and are presented by problem area in ascending order from left to right on the horizontal access.

The benefit/cost score is shown as a bar chart in blue. Additionally, the cost per gallon of flood reduction is included as a line on a logarithmic scale. This metric provides an alternative cost-based method for ranking projects. It is important to remember that the best projects will have a high benefit/cost score but a low cost per gallon of flood reduction.



FIGURE 5-2  
Benefit/Cost Chart for High-priority Problem Areas 601 through 603  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*







## 5.4 Watershed-wide Alternatives

Three watershed-wide alternatives were developed for Strawberry Run. Each watershed-wide alternative was aimed at resolving capacity-related issues while also meeting a second goal: including maximizing cost-efficiency or benefit cost, or targeting the highest-priority problems. The three alternatives examined include:

- Alternative 1: Most cost-effective solution for each problem area (lowest dollar-per-gallon of flood reduction)
- Alternative 2: Best benefit/cost ratio for each problem area (highest benefit/cost ratio)
- Alternative 3: Combination of best projects to resolve the highest-priority problem areas

Projects were selected for each of the watershed-wide alternatives based on the five individual technology-specific modeling results (Conveyance, Storage, and Low GI, Medium GI, and High GI implementation). A new model including the selected projects was run for each alternative. Results for the watershed-wide model runs are presented in Section 5.4.4 and 5.4.5.

### 5.4.1 Alternative 1: Cost Efficiency

The first alternative focused on providing the best cost efficiency in each problem area. After removing projects that did not meet the minimum flood reduction threshold of 22 percent, the remaining projects were ranked by cost-per-gallon of flood reduction within each problem area in ascending order. The highest-ranked project, which was the project with the lowest cost-per-gallon of flood reduction, was selected for each problem area. Table 5-3 shows the selected project for each problem area, including two storage solutions and one conveyance solution. Model results are summarized in Table 5-6 and presented on Figure 5-3.

The model results of this alternative show significant reduction in flooding in the high-priority problem areas, with 86 percent of the problem area flooding being reduced. The conveyance and storage solutions in this alternative eliminated flooding in two of the three high-priority problem areas. The Storage solution in Problem Area 602 had the smallest impact on flood volume reduction (34 percent). This alternative results in total flood volume reduction of 86 percent in the high-priority problem areas.

TABLE 5-3  
Selected Projects for Watershed-wide Alternative 1: Cost Efficiency  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Solution Technology	Project Name	Capital Cost (\$M)	Benefit-Cost Ratio	Flood Volume Reduction (MG)	Flood Volume Reduction (%)	Cost/Gallon of Flood Reduction (\$/gal)
601	Storage	STOR-601	\$0.16	173.7	0.038	100	\$4.14
602	Storage	STOR-602	\$0.05	346.7	0.006	34	\$8.38
603	Conveyance	CONV-603	\$0.35	135.9	0.020	100	\$17.12
<b>Total</b>			<b>\$0.55</b>		<b>0.064</b>	<b>86<sup>a</sup></b>	<b>\$8.59</b>

Note:

Results presented in this table are based on five separate technology-based model runs (Conveyance, Storage, and Low, Medium, and High GI).

<sup>a</sup> Existing flood volume for Problem Areas 601 through 603 is 0.074 MG.

### 5.4.2 Alternative 2: Benefit/Cost

The second alternative focused on providing the best benefit/cost in each problem area. After removing projects that did not meet the minimum flood reduction threshold of 22 percent, the remaining projects were ranked by benefit/cost in descending order. The highest-ranked project in each of the three problem areas, which was the project with the highest benefit/cost score, was selected. Table 5-4 shows the selected project for each problem area. This alternative consisted of Low and Medium GI projects. Model results are summarized in Table 5-6 and presented on Figure 5-4. Unlike Alternative 1, flooding is not eliminated, but significant reductions are achieved.

This alternative results in a total flood volume reduction of 57 percent across the three high-priority problem areas.

TABLE 5-4  
Selected Projects for Watershed-wide Alternative 2: Benefit/Cost  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Solution Technology	Project Name	Capital Cost (\$M)	Benefit-Cost Ratio	Flood Volume Reduction (MG)	Flood Volume Reduction (%)	Cost/Gallon of Flood Reduction (\$/gal)
601	Medium GI	MGI-601	\$0.20	244.8	0.018	46	\$11.28
602	Low GI	LGI-602	\$0.08	476.2	0.008	49	\$10.31
603	Medium GI	MGI-603	\$0.34	161.0	0.016	82	\$21.56
<b>Total</b>			<b>\$0.62</b>		<b>0.042</b>	<b>57<sup>a</sup></b>	<b>\$14.88</b>

Note:

Results presented in this table are based on five separate technology-based model runs (Conveyance, Storage, and Low, Med, and High GI).

<sup>a</sup> Existing flood volume for Problem Areas 601 through 603 is 0.074 MG.

### 5.4.3 Alternative 3: Highest-priority Problems

The third alternative focused on resolving the highest-priority problems by combining multiple solutions within a problem area, with less emphasis on cost benefit or efficiency. This alternative also overrides the minimum threshold of 22 percent flood reduction because the goal is to eliminate as much flooding as possible from the highest-priority problem areas. Therefore, a conveyance or storage project that offered substantial flood reduction when combined with a project such as low GI, which offered less than 22 percent flood reduction, could eliminate flooding within a problem area. The best combination of solutions in terms of cost efficiency, benefit/cost, and overall flood reduction were compiled to attempt to resolve the worst problem areas. Because three projects were recommended in Alternatives 1 and 2 (one per project area), three projects were selected for Alternative 3 to keep all three alternatives relatively consistent in scale. This alternative consisted two storage projects and one Low GI project.

Table 5-5 shows the selected project(s) for each problem area and the results based on the three individual technology-based model runs. Model results are summarized in Table 5-6 and shown on Figure 5-5. This alternative results in a total flood volume reduction of 70 percent across the two high-priority problem areas addressed in this alternative.

TABLE 5-5  
Selected Projects for Watershed-wide Alternative 3: Highest-priority Problems  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

Problem Area ID	Solution Technology	Project Name	Capital Cost (\$M)	Benefit-Cost Ratio	Flood Volume Reduction (MG)	Flood Volume Reduction (%)	Cost/Gallon of Flood Reduction (\$/gal)
601	Storage	STOR-601	\$0.16	173.7	0.038	100	\$4.14
602	Storage	STOR-602	\$0.05	346.7	0.006	34	\$8.96
602	Low GI	LGI-602	\$0.08	476.2	0.008	49	\$14.70
<b>Total</b>			<b>\$0.27</b>		<b>0.052</b>	<b>70<sup>a</sup></b>	<b>\$5.19</b>

Note:

Results presented in this table are based on five separate technology-based model runs (Conveyance, Storage, and Low, Medium, and High GI).

<sup>a</sup> Existing flood volume for Problem Areas 601 through 603 is 0.074 MG.

#### 5.4.4 Modeling Results

Table 5-6 provides a summary of the hydraulic model results for the three watershed-wide alternatives. All three alternatives provide significant flood reduction in the high-priority problem areas, but the impact of the alternatives on the watershed as a whole is limited. This is because the high-priority problems are located near stream outfalls with a relatively small portion of the system upstream and downstream of them. Therefore, changes made in the high-priority problem areas do not have a limited impact on the rest of the watershed. Maps comparing the model results are presented on Figures 5-3 through 5-5.

Each of the alternatives analyzed leaves areas with flooding (as shown by red lines on the maps), largely because those areas are outside the boundaries of the high-priority problem areas. These areas were not addressed by solutions because they were either flooding at isolated structures, or did not score high based on the problem area scoring criteria.

TABLE 5-6

## Summary of Watershed-wide Alternative Model Results

*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Existing Conditions Results				Alternative 1 Best Cost Efficiency				Alternative 2 Best Benefit/Cost Ratio				Alternative 3 Highest-priority Problems			
	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>	Conduit Length (LF)	Percent of Total Length (%)	Total Duration (hrs)	Total Volume (ft <sup>3</sup> ) <sup>b</sup>
Sufficient Capacity	13,802	54	-	-	14,670	58	-	-	14,979	59	-	-	14,506	57	-	-
Surcharged <sup>a</sup>	4,308	17	35	-	4,267	17	31	-	4,325	17	33	-	4,598	18	32	-
Insufficient Freeboard	3,392	13	-	-	3,254	13	-	-	2,530	10	-	-	2,982	12	-	-
Flooded	3,877	15	13	67,657	3,188	13	11	59,021	3,544	14	12	60,903	3,293	13	12	61,823

## Notes:

Results presented for pipe segments are based on capacity at upstream end of pipe.

<sup>a</sup> Duration of surcharged flow includes time during which conduits are surcharged, have insufficient freeboard or are flooded at upstream end only.<sup>b</sup> Flooded volume includes volume flooded at upstream end of the conduit.

FIGURE 5-3

Alternative 1: Cost-efficiency Model Results

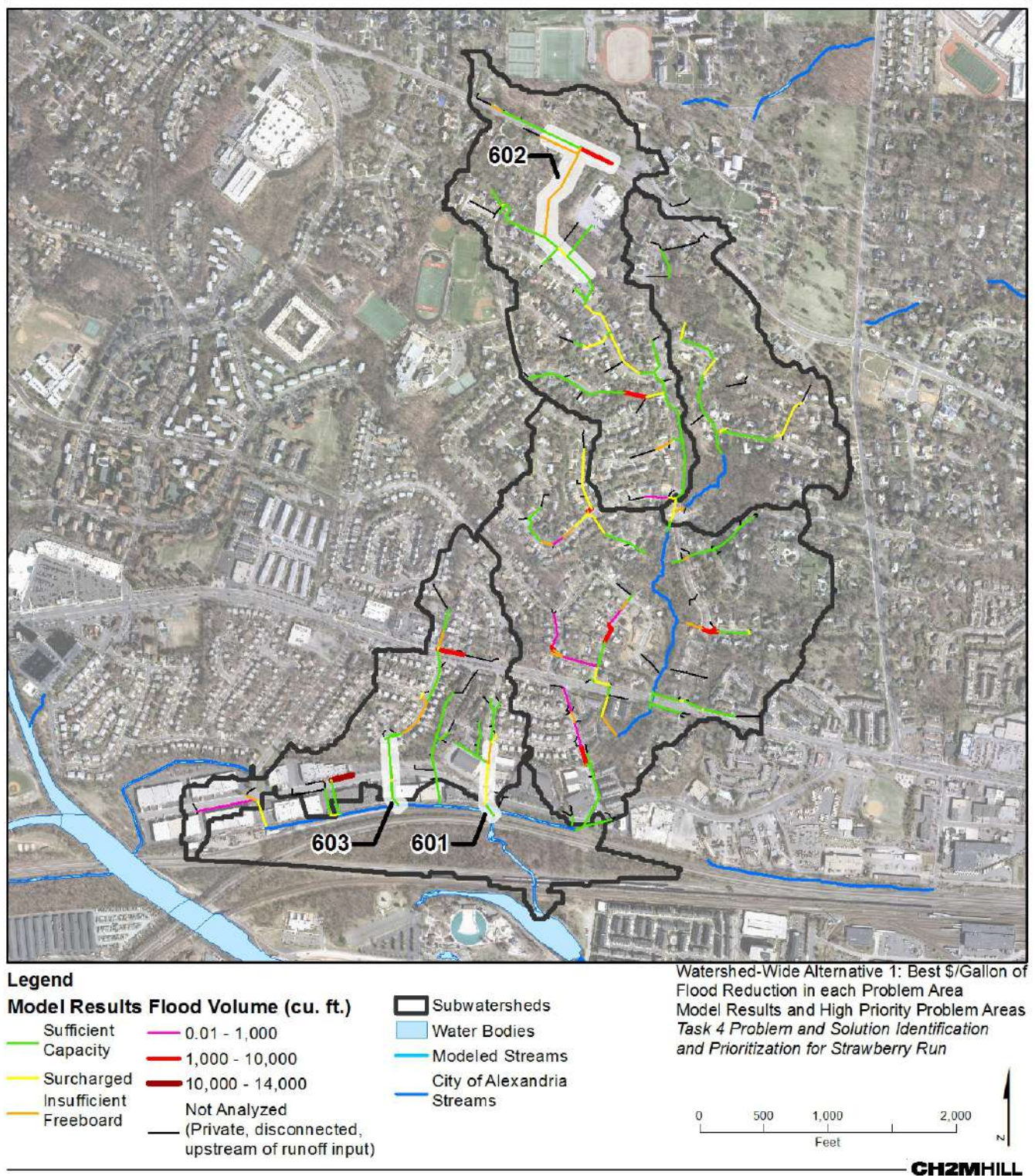
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





FIGURE 5-4

Alternative 2: Benefit/Cost Model Results

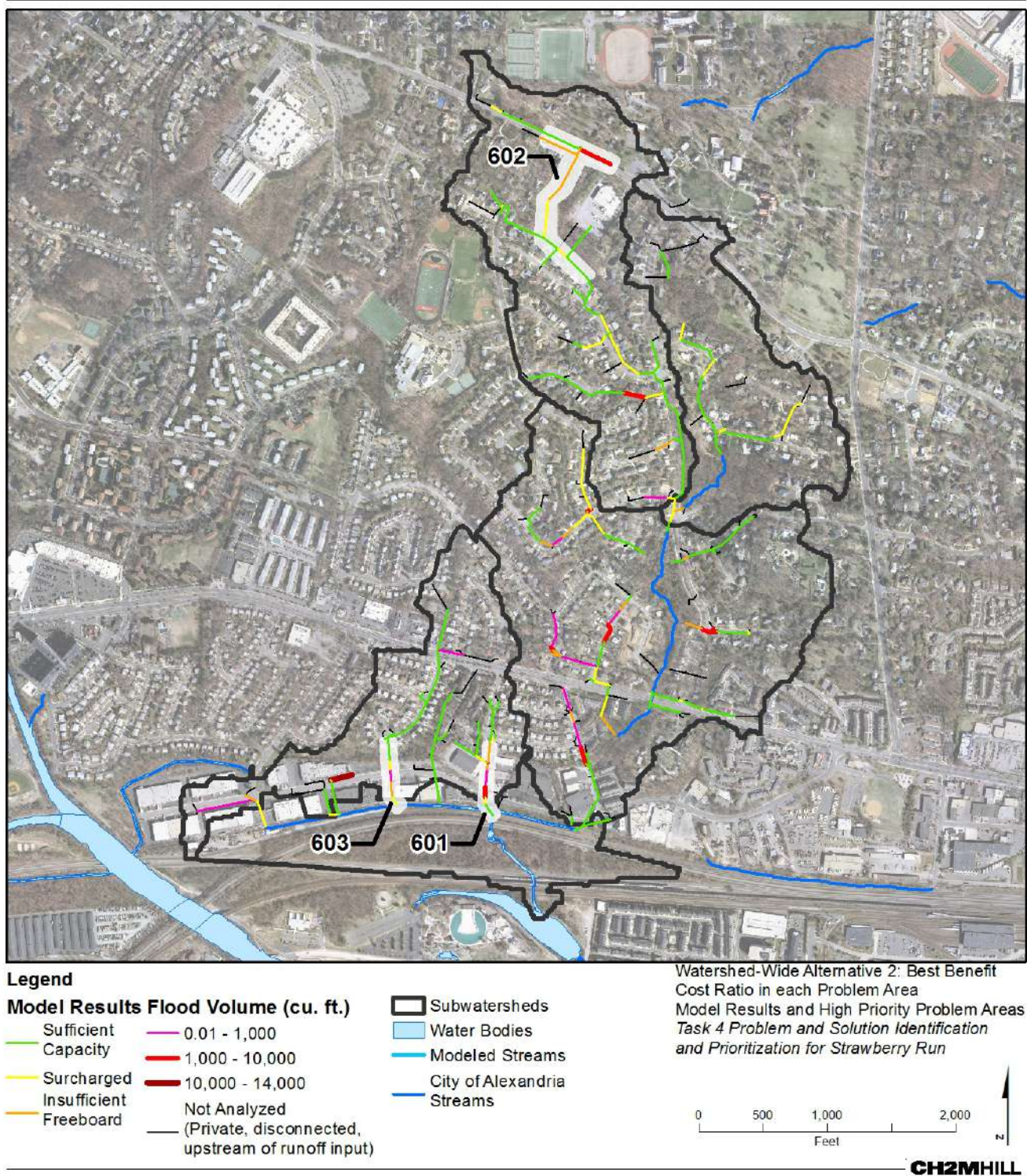
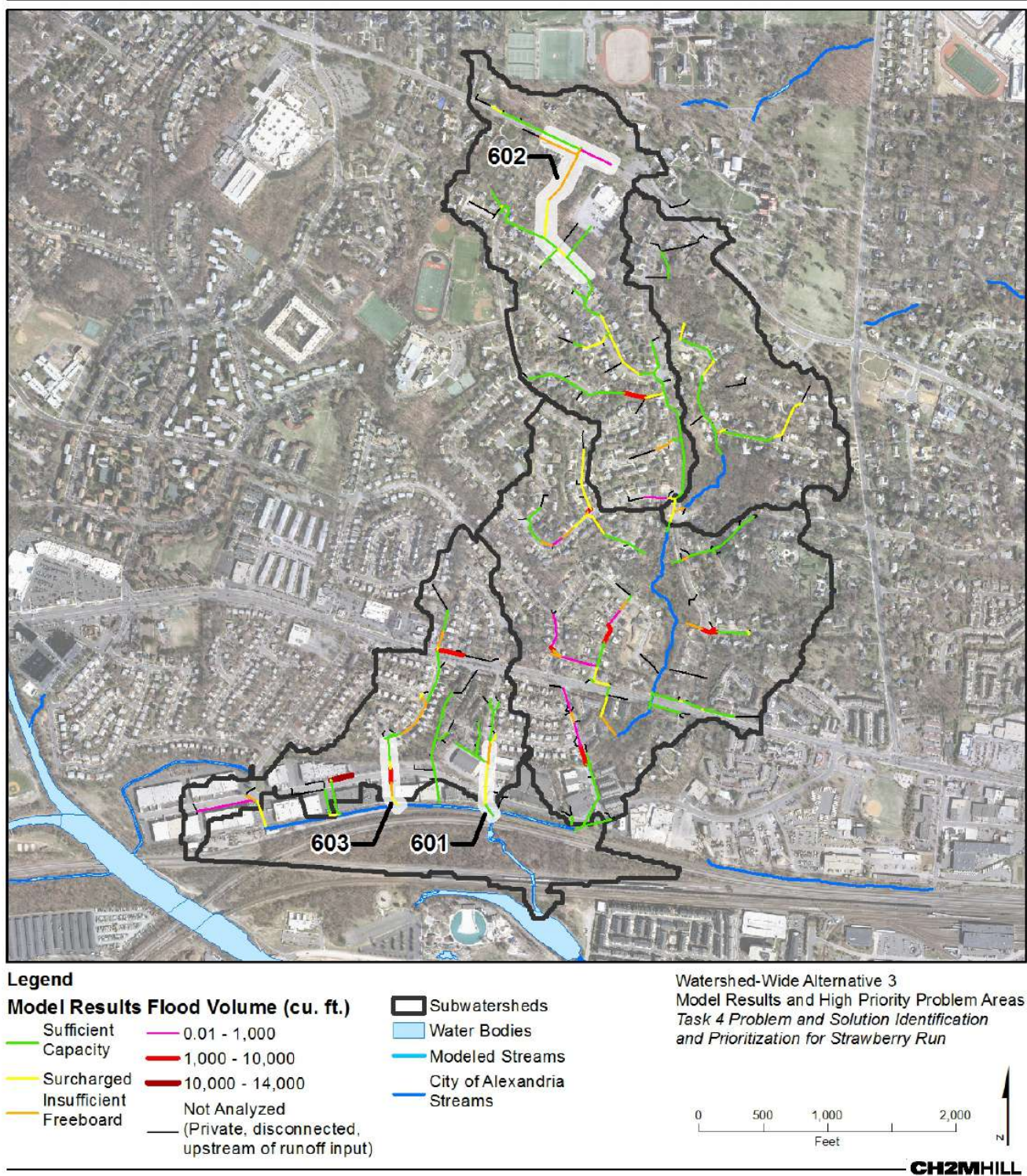
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





FIGURE 5-5

Alternative 3: Highest-priority Problem Areas Model Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





### 5.4.5 Scoring and Prioritization Results

The results for each alternative reflect the objective upon which it was built. A summary of the results is provided in Table 5-7. A model was run for each of the alternatives, so the alternative specific results presented in Table 5-7 may differ slightly from the results generated from the technology specific model runs used to evaluate each solution type.

The three watershed-wide alternatives range in total cost from \$270,000 to \$620,000 and eliminate between 57 and 86 percent of flooding in the high-priority problem areas. Alternative 1 eliminates the most flooding of the three watershed-wide alternatives, but has the lowest benefit/cost ratio of the 3 alternatives. Alternative 2 has the highest benefit score, but also has the highest cost and cost/gallon of flood reduction. Alternative 3, while only addressing two of the three high-priority problem areas, has the lowest cost and cost/gallon of flood reduction and highest benefit cost ratio while eliminating 70 percent of flooding in the three high-priority problem areas and 88 percent of flooding in problem areas 601 and 602. For those reasons, Alternative 3 is the recommended alternative for the Strawberry Run watershed.

TABLE 5-7

Watershed-wide Alternatives Scoring and Prioritization Results

*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Alternative 1 - Best Cost Efficiency	Alternative 2 - Best Benefit/Cost Ratio	Alternative 3 – Highest-priority Problems
Total Cost (\$ Millions)	\$0.55	\$0.62	\$0.27
Total Benefit Score	90.4	143.7	88.2
Overall Benefit/Cost	164.4	231.8	321.7
Total Flood Reduction (MG)	0.064	0.042	0.047
\$/Gallon of Flood Reduction	\$8.58	\$14.76	\$5.77

Note:

Results presented in this table are based on watershed-wide alternative models that include the selected projects documented in sections 5.4.1-5.4.3.

When developing a capital improvement plan, the benefit cost or cost efficiency (\$/gallon of flood reduction) are typically used to guide the order in which projects are implemented. Prioritization results for the three watershed-wide alternatives are presented in Figures 5-6 through 5-8. The top chart shows the benefit cost ratio and the cumulative capital cost of the alternative. The solutions are provided in order of decreasing benefit cost ratio; solutions with the greatest benefit cost ratio are presented on the left and solutions with the lowest benefit cost ratio are presented on the right.

The bottom chart shows the benefit/cost ratio for each solution in the watershed-wide alternative in order of increasing cost/gallon of flood reduction. Both charts show the cumulative capital cost plotted on the secondary axis. The solutions on both charts are named by the technology: Conveyance (CONV), Storage (STOR), Low GI (LGI), Medium GI (MGI), or High GI (HGI), and the problem area number.





FIGURE 5-6

## Alternative 1: Best Cost Efficiency Prioritization Results

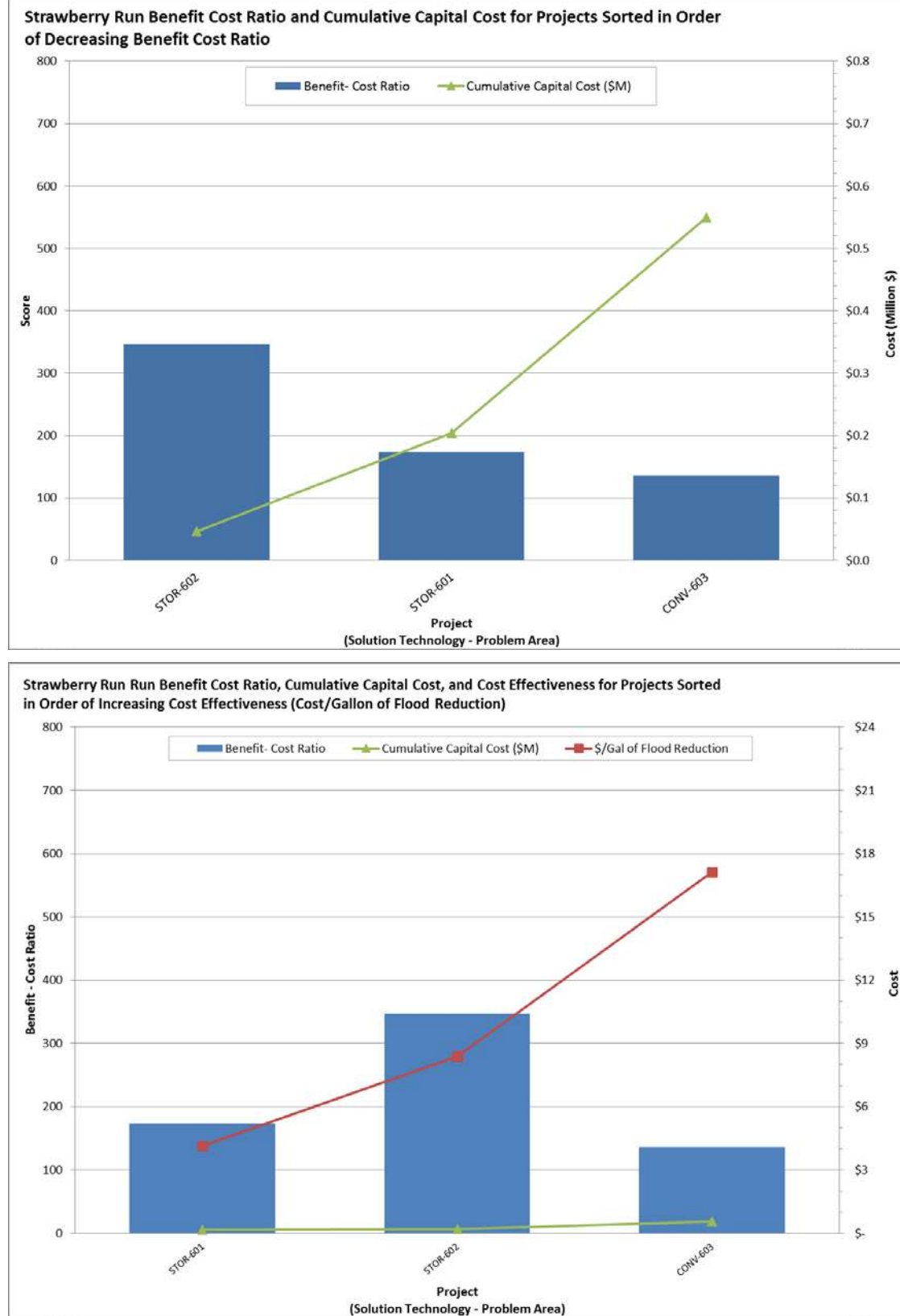
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*



FIGURE 5-7

## Alternative 2: Best Benefit/Cost Ratio Prioritization Results

## City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run

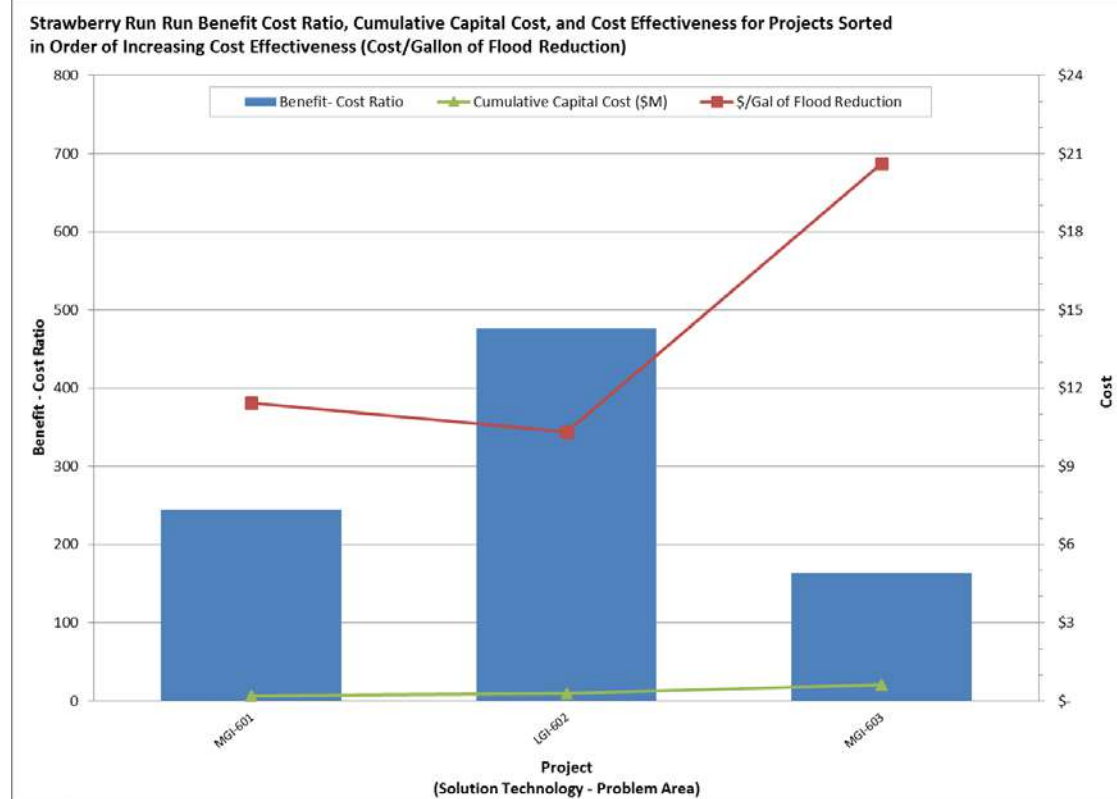
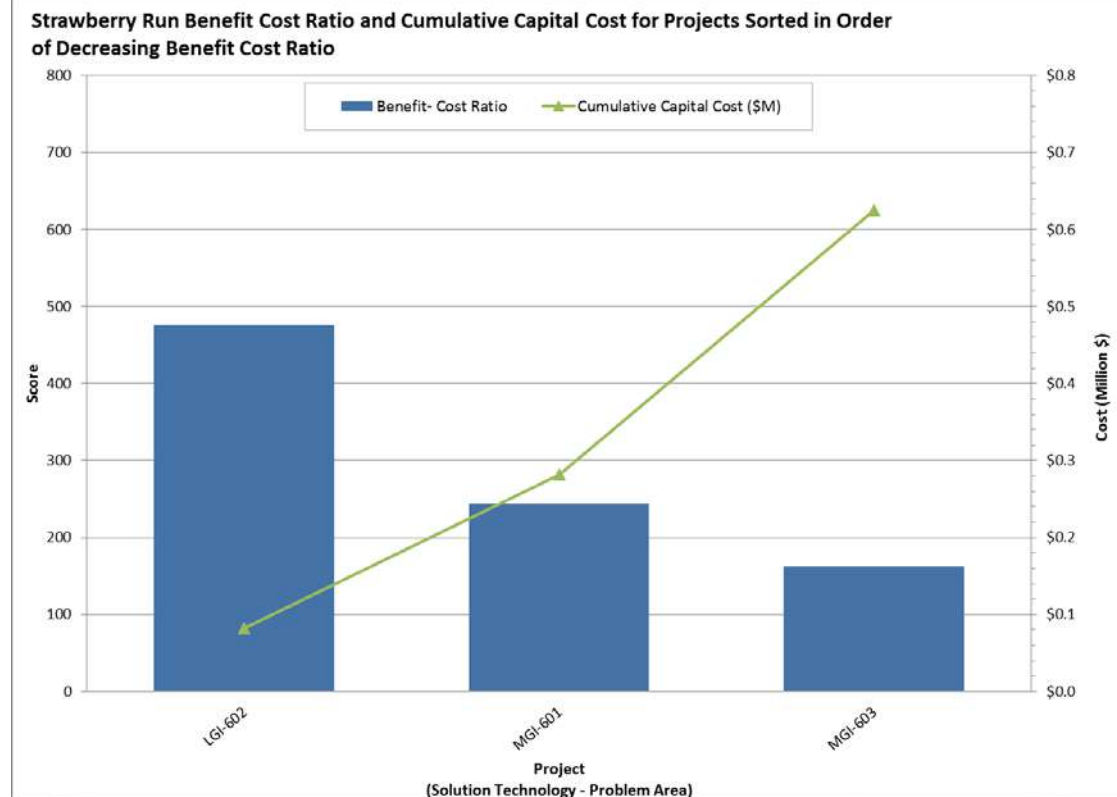
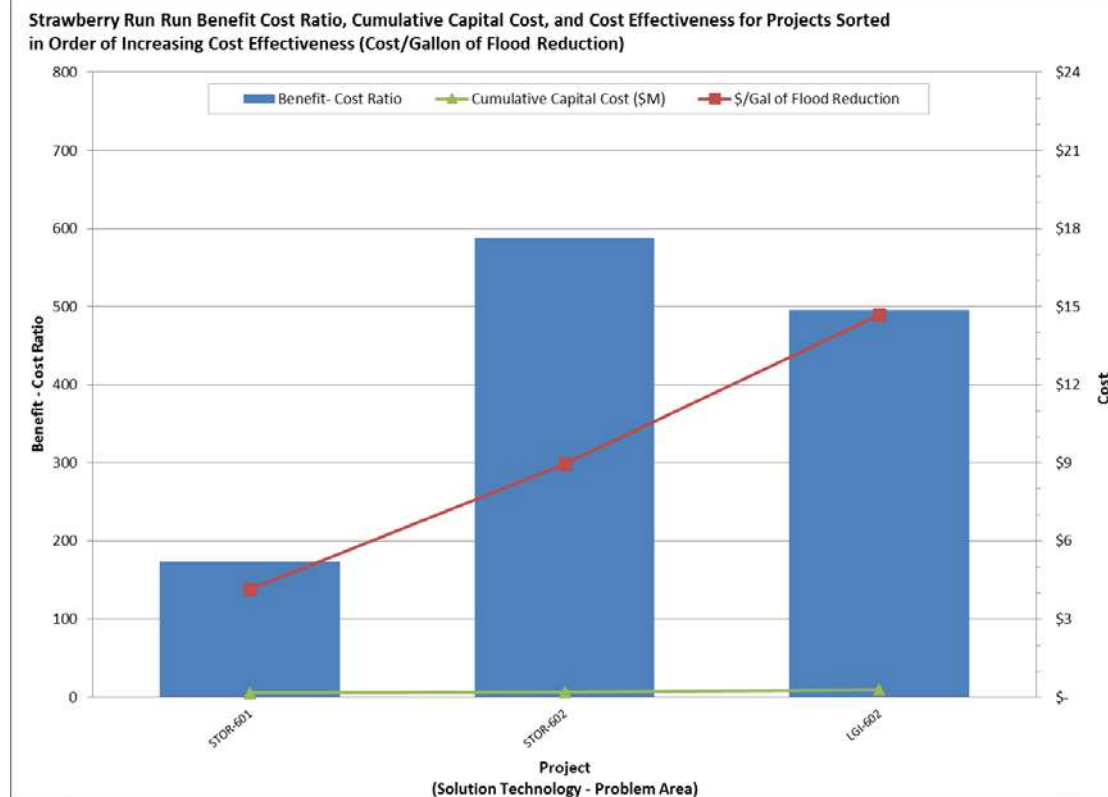
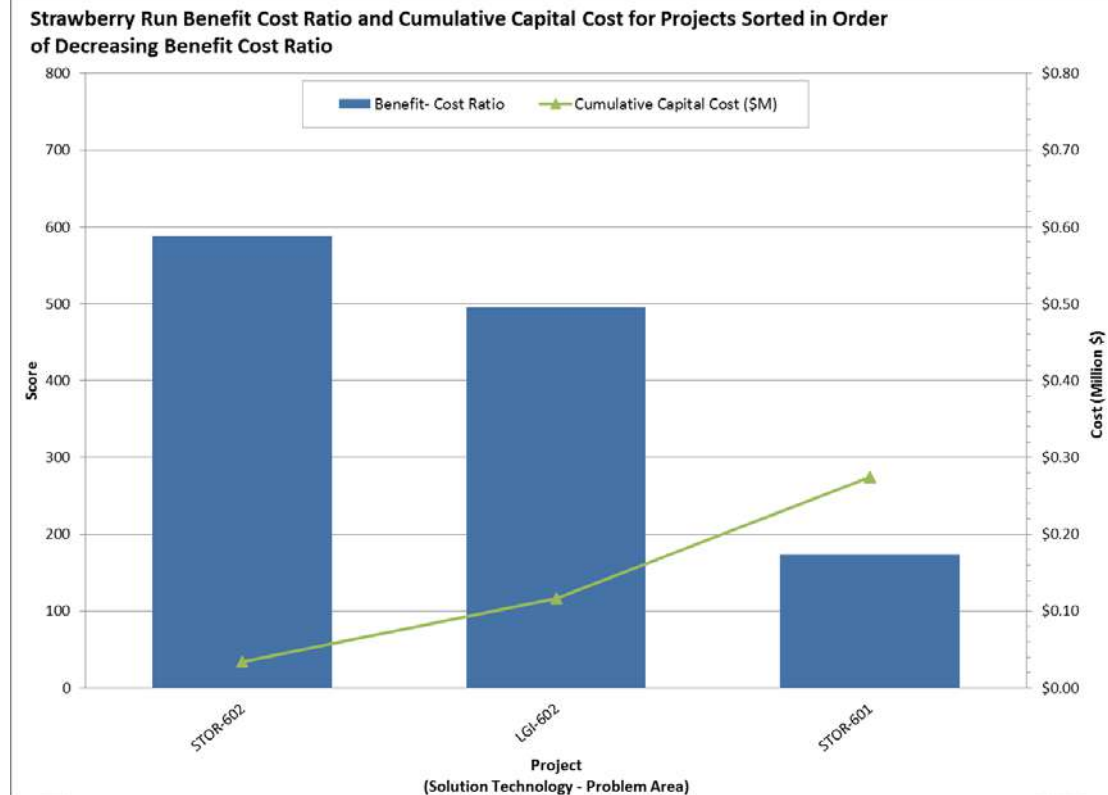






FIGURE 5-8

Alternative 3: Highest-priority Problems Prioritization Results

*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*



## Summary

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The objectives of this phase of the study were to: 1) identify and prioritize capacity problems based on modeling results from Task 2 of this project, and 2) develop and prioritize solutions to address those problems. The first objective was accomplished in two steps. The first step included evaluating each stormwater junction in the drainage network using a scoring system to identify problems based on several criteria, including the severity of flooding, proximity to critical infrastructure and roadways, identification of problems by City staff and the public, and opportunity for overland relief. In the next step of this objective, high-scoring junctions (that is, higher-priority problems) were grouped together to form high-priority problem areas. In total, three high-priority problem areas were identified in the Strawberry Run watershed.

The second objective involved developing and prioritizing solutions to address capacity limitations within the three high-priority problem areas. To accomplish this objective, several strategies involving different technologies were examined, including improving conveyance by increasing hydraulic capacity, reducing capacity limitations by adding distributed storage to the system, and reducing stormwater inflows by implementing GI. Each of these strategies required a different modeling approach. Conveyance improvements were modeled by increasing pipe diameter in key locations within the problem area, storage was added as storage nodes based on a preliminary siting exercise, and GI was modeled as a reduction in impervious area at three different implementation levels: high, medium, and low. A single model run was set up and run for each strategy addressing all three high-priority problem areas and the results were compiled for the alternatives and prioritization evaluation. Solutions were evaluated based on several criteria, including drainage improvement/flood reduction, environmental compliance, sustainability and social benefits, asset management and maintenance implications, constructability, and public acceptance. Planning-level capital costs were developed for each solution to facilitate a benefit cost analysis and prioritization process.

The results of the solution identification and prioritization analysis show the following results for Strawberry Run:

- In terms of solution technology performance:
  - High and Medium GI solutions generally have the greatest overall benefit in Strawberry Run.
  - Conveyance, Storage, High GI, and Medium GI solutions all provide significant flood reduction for the problem areas analyzed.
- In terms of costs:
  - A low level of GI implementation generally has the greatest benefit/cost score, but did not usually meet the minimum threshold for flood reduction.
  - The cost per gallon of flood reduction appears to be highly dependent on the problem area, but in general, Conveyance, Storage, and Low GI projects provide the most economical stormwater volume reduction in terms of dollars per gallon of flood reduction within a high-priority problem area.

Three watershed-wide alternatives were developed, including:

- Alternative 1: Most cost-effective solution for each problem area (lowest dollar-per-gallon of flood reduction)
- Alternative 2: Best benefit/cost ratio for each problem area (highest benefit/cost ratio)
- Alternative 3: Combination of best projects to resolve the worst problem areas

In Strawberry Run, the three problem areas are well spread out across the watershed and discharge to separate outfalls. For this reason, increasing the capacity to alleviate flooding in one problem area did not increase the flooding in other problem areas. However, the flood volume in the problem areas is small enough that storage and GI projects developed in the project were able to eliminate flooding at a lower cost than conveyance projects. For this reason, storage and GI projects make up the majority of projects in the watershed-wide scenarios analyzed in this study.

A summary of the watershed-wide results is provided in Table 6-1.

TABLE 6-1  
Watershed-wide Alternatives Scoring and Prioritization Results  
*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*

	Alternative 1 - Best Cost Efficiency	Alternative 2 - Best Benefit/Cost Ratio	Alternative 3 – Highest-priority Problems
Total Cost (\$ Millions)	\$0.55	\$0.62	\$0.27
Total Benefit Score	90.4	143.7	88.2
Overall Benefit/Cost	164.4	231.8	321.7
Total Flood Reduction (million gallons)	0.064	0.042	0.047
\$/Gallon of Flood Reduction	\$8.58	\$14.76	\$5.77

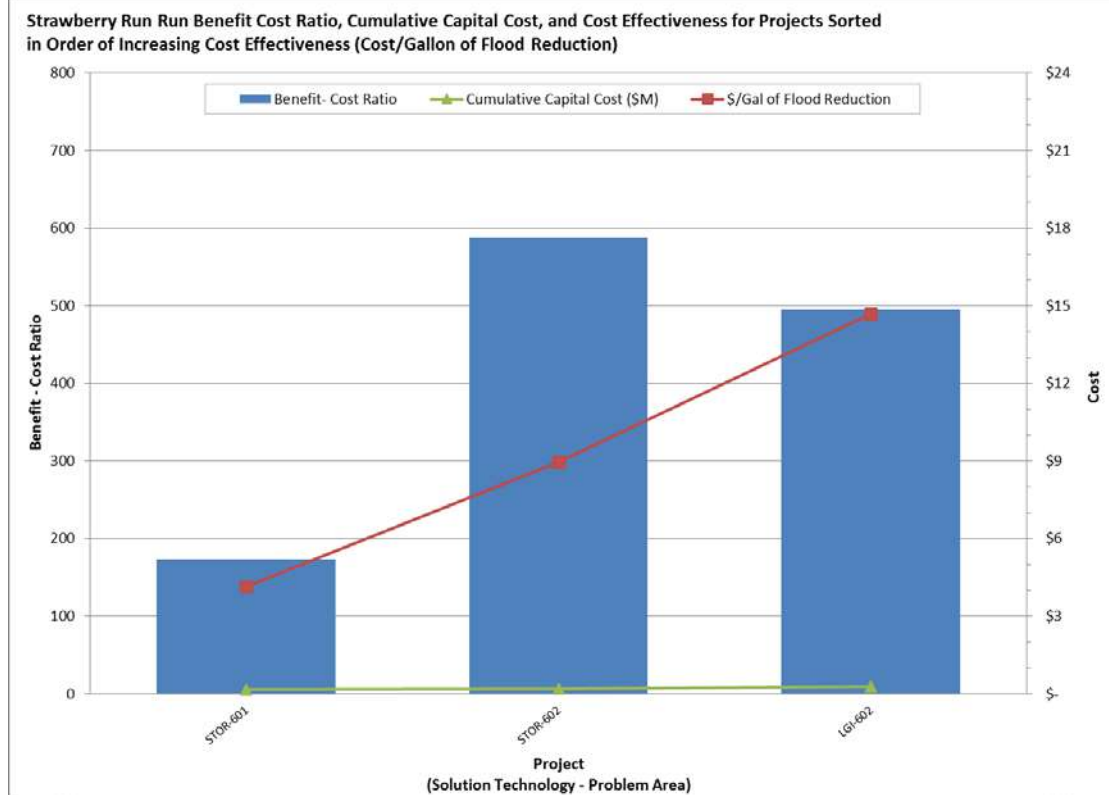
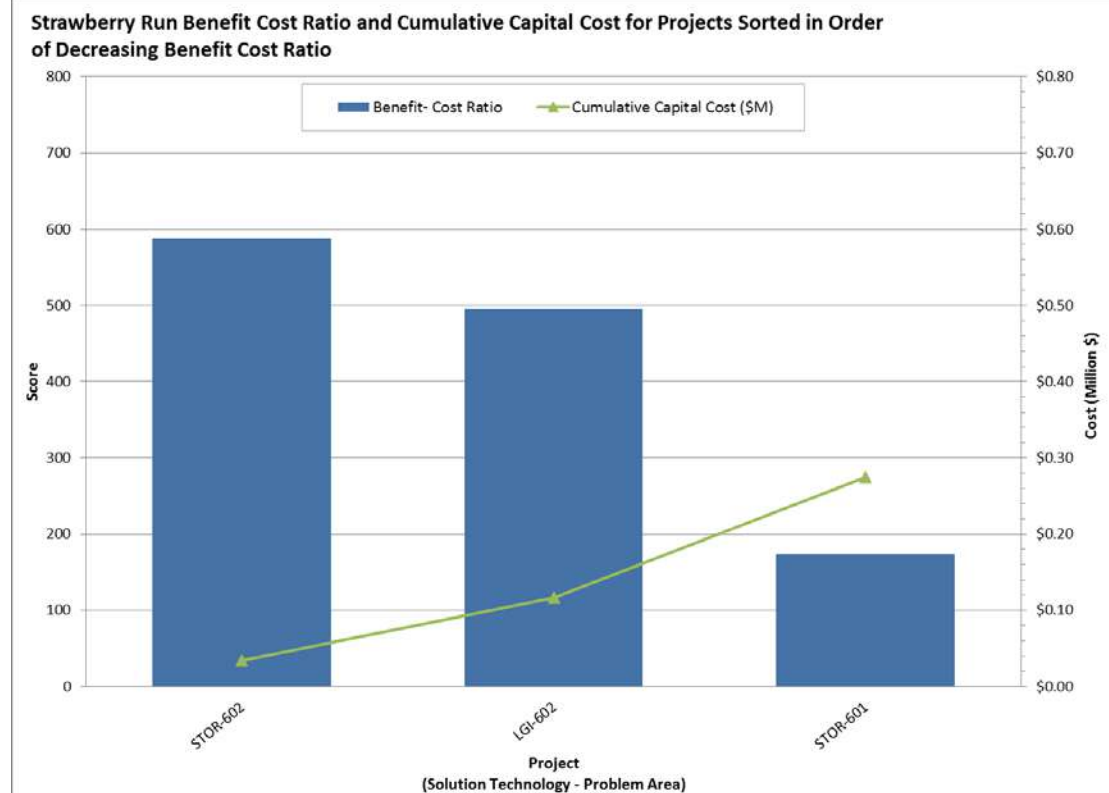
The three watershed-wide alternatives range in total cost from \$270,000 to \$620,000 and eliminate between 57 and 86 percent of flooding in the high-priority problem areas. Alternative 1 eliminates the most flooding of the three watershed-wide alternatives, but has the lowest benefit/cost ratio of the three alternatives. Alternative 2 has the highest benefit score, but also has the highest cost and cost/gallon of flood reduction. Alternative 3, while only addressing two of the three high-priority problem areas, has the lowest cost and cost/gallon of flood reduction and highest benefit cost ratio while eliminating 88 percent of flooding in high-priority problem areas 601 and 602. For those reasons, Alternative 3 is the recommended alternative for the Strawberry Run watershed. Two suggested prioritization of watershed-wide Alternative 3 projects are provided in Figure 6-1; projects can be prioritized either based on overall benefit/cost ratio or cost efficiency (cost per gallon of flood reduction).

It should be noted that the model does not include analysis on private property, but applies assumed runoff loads as inputs to the public conveyance system. The City chose not to include existing private or most public stormwater management facilities (e.g., detention and retention ponds) upstream of the modeled collection system because of the limited available information on these facilities and a concern that the facilities may not be performing as designed. When the City moves forward into detailed evaluation and design of selected projects, it will be important to fully evaluate and account for the benefits of any existing stormwater management facilities.

The hydraulic modeling results and costs presented in this report should be reviewed with the understanding that several assumptions were made to fill data gaps in the hydraulic model, and proposed solutions and costs were developed on a planning level.

FIGURE 6-1

Alternative 3: Highest-priority Problems Prioritization Results

*City of Alexandria Storm Sewer Capacity Analysis – Strawberry Run*





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## Appendix A

### Conveyance Solutions

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## Appendix A - Conveyance Solutions

Summary of Conveyance Solutions Developed for Strawberry Run High-Priority Problem Areas

Problem Area	FacilityID	Upstream Node Name	Downstream Node Name	Length ft	Proposed Shape	Existing Diameter/ Height (ft) x Width (ft)	Proposed Diameter/ Height (ft) x Width (ft)	Conduit Slope	Number of Barrels	Roughness
601	000933STMP	001894IN	001904IN	48.866	CIRCULAR	1.5	2.5	5.464	1	0.013
601	002176STMP	001904IN	000135IO	110.971	CIRCULAR	1.5	2	7.957	1	0.013
601	002775STMP	000613SMH	000614SMH	28.246	CIRCULAR	1.5	2	3.753	1	0.013
601	002776STMP	000614SMH	000615SMH	144.054	CIRCULAR	1.5	2.5	2.742	1	0.013
601	002778STMP	000615SMH	001894IN	80.399	CIRCULAR	1.5	2.5	1.468	1	0.013
601	003325STMP	000610SMH	000611SMH	94.763	CIRCULAR	1.25	1.5	6.494	1	0.013
601	003326STMP	000611SMH	000612SMH	51.906	CIRCULAR	1.25	1.5	8.729	1	0.013
601	003327STMP	000612SMH	000613SMH	26.759	CIRCULAR	1.25	2	5.456	1	0.013
602	002887STMP	002862IN	002861IN	267.292	CIRCULAR	1.25	2	0.802	1	0.013
602	002890STMP	002861IN	002864IN	51.307	CIRCULAR	1.25	2.5	1.072	1	0.013
602	002891STMP	000889SMH	002865IN	281.651	CIRCULAR	1.75	2.5	5.248	1	0.013
602	002892STMP	002865IN	002867IN	130.897	CIRCULAR	2	2.5	3.812	1	0.013
602	002894STMP	002866IN	000891SMH	139.196	CIRCULAR	3	3.5	1.789	1	0.013
602	002895STMP	002867IN	002866IN	291.681	CIRCULAR	2	2.5	3.74	1	0.013
602	002898STMP	002870IN	002871IN	235.857	CIRCULAR	3.5	4.5	1.208	1	0.013
602	002899STMP	000891SMH	002870IN	92.223	CIRCULAR	3	4	1.702	1	0.013
602	004570STMP	002852IN	002864IN	230.02	CIRCULAR	1.25	2	0.886	1	0.013
602	004579STMP	002854IN	002852IN	90.93	CIRCULAR	1.25	1.5	1.312	1	0.013
602	004581STMP	002861IN	002863IN	51.331	CIRCULAR	1.25	2.5	1.013	1	0.013
603	002035STMP	001832IN	001834IN	118.814	CIRCULAR	3	4.5	0.421	1	0.013
603	002785STMP	001834IN	000134IO	74.873	CIRCULAR	3	6	-0.12	1	0.013
603	002974STMP	001847IN	000616SMH	71.578	CIRCULAR	3	4.5	0.535	1	0.013
603	002981STMP	000626SMH	001893IN	186.478	CIRCULAR	2.5	3	3.614	1	0.013
603	014708STMP	001893IN	001847IN	60.997	CIRCULAR	3	4.5	0.536	1	0.013



## Appendix B

### Storage Solutions

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Appendix B - Storage Solutions

Summary of Storage Solutions Developed for Strawberry Run High-Priority Problem Areas

Problem Area	Storage ID	Overflow Node	Discharge Node	Storage Area (ac)	Storage Area (ft <sup>2</sup> )	Overflow Weir Crest	Overflow Weir Crown	Storage Invert Elevation (ft)	Storage Rim Elevation (ft)	Storage Depth (ft)	Storage Volume (ft <sup>3</sup> )	Notes
601	STOR_601	000613SMH	001904IN	0.05	2,108	54.22	63.32	44.00	54	10.00	21,080	Private Property - Green strip adjacent to Wheeler Ave right-of-way; designated as commercial vacant land
602	STOR_602	002861IN	000889SMH	0.09	4,031	262.43	263.83	256.60	264	5.83	23,498	Private property - Episcopal Seminary open space; trees in the area, so limited clearing may be necessary
603	STOR_603	001847IN	000616SMH	0.13	5,798	41.71	44.71	38.33	46	4.20	24,352	Private Property - parking lot designated as commercial vacant land; 3.4 ft of depth is available below the weir crest, volume can be utilized up to storage rim, which is ~10 ft above storage invert, or easily up to 4.2 ft with minimal surcharging in system





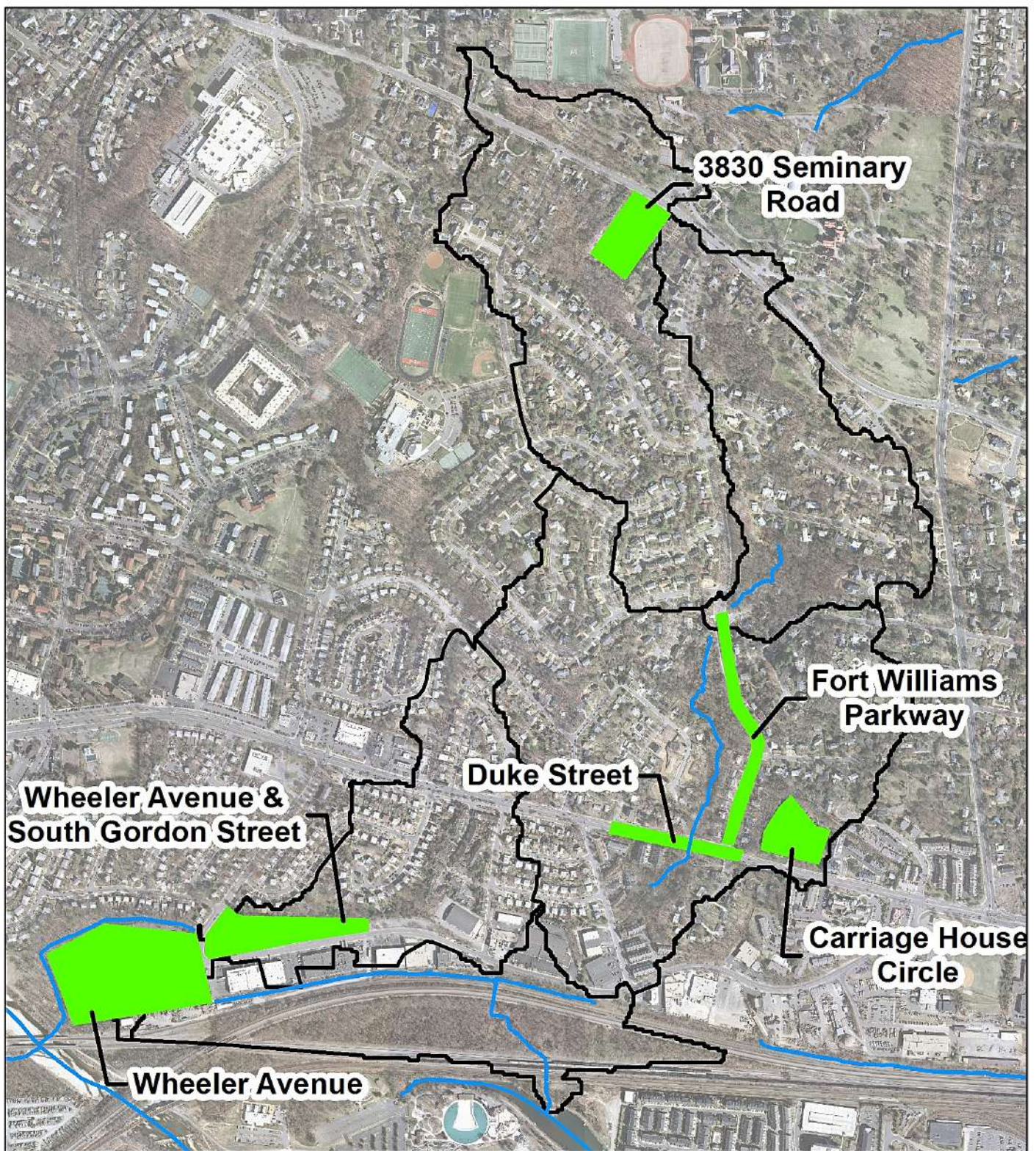
## Appendix C

### Green Infrastructure Concept Plans

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#### LEGEND

- Concept Plan Locations
- Subwatersheds
- City of Alexandria Streams

#### Task 4 Green Infrastructure Program Concepts for Strawberry Run

Task 4 - Identify Problems and Develop Solutions  
City of Alexandria Storm Sewer Capacity Analysis

0 750 1,500 Feet







# Potential Sites for Task 4 Concept Development in Strawberry Run

PREPARED FOR: City of Alexandria TE&S Department

PREPARED BY: CH2M HILL

COPY TO: File

DATE: August 13, 2015

PROJECT NUMBER: 240027

The following is documentation of the sites identified as potential locations for green infrastructure (GI) concept development in Strawberry Run. For each site a program and the elements of the program are identified with field notes as well as pros and cons of GI implementation. Sites are described with the easternmost site in Strawberry Run first, moving west across the watershed. A map of the watershed and all potential sites, as well as a detailed map of each individual site, is provided in Appendix A for reference. No schools are located in the Strawberry Run watershed therefore a Green School program was not developed for the watershed.

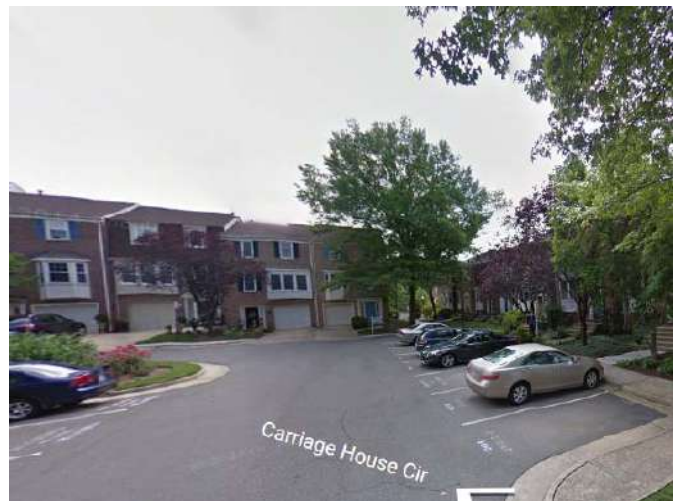
## Carriage House Circle

### Area for Bioretention/Planter and Tree Filter Box by Mailbox



Source: Google Maps Street View™

### Parking Spaces for Porous Pavement



Source: Google Maps Street View™

**Program Type:** Green Parking,

**GI Concepts:** Bioretention/Planters. Porous Pavement, Tree Filter Box

### Field Notes:

- A circle of townhouses surrounding a large pervious parking island area that is about 6 feet above street level. The topography does not allow drainage to the pervious area. No opportunity to use this large portion of the pervious area for GI installation. Impervious surfaces are directly connected to storm sewer.
- A combination of planters or bioretention and porous pavement installed at the northern corner of the pervious parking island area would improve infiltration of runoff from roof and road pavement at the north end of the site.



- Excess imperviousness on the concrete pad containing the mailbox could be converted to a planter to improve infiltration of runoff from the nearby parking spots.
- A bioretention could be installed between the rows of houses on the west side of the site to receive and infiltrate runoff from parking and road pavements.
- Porous Pavement in the south east corner parking spaces, potential for infiltration of roof and roadway runoff from the east of the site.
- Existing curb inlets located at the entrance to the site could be fitted with tree filter boxes to improve roadway infiltration.

**Pros:**

- Drainage pattern of the site would aid distributed GI implementation.

**Cons:**

- Slopes/ topography may limit GI technology options.
- Requires coordination with private property owners

## Fort Williams Pkwy.

**Wide Median at South End of Fort Williams Pkwy**



**Potential Site at Dearborn Place for Bioretention**



**Program Type:** Green Street

**GI Concepts:** Planters/Bioretention

**Field Notes:**

- Majority of the roadway drains southwards where there is a wide median with gentle slope for potential bioretention implementation.
- The northbound lane is crested in the middle of the road so that only half of the road way drains towards the median. A trench drain could be used as runoff diversion structure to divert runoff from both halves of the north bound roadway towards the median for treatment.
- Bioretention could be installed at the east corner of Fort Williams Pkwy and Duke Street to receive runoff from the east have of the north bound roadway.
- The South bound roadway drains away from the median so a runoff diversion in the form of trench drain may be used to divert runoff from the south bound lane to the median for treatment.

- Bioretention could be installed adjacent to the south bound lane between Trinity Dr. and Tupelo Place. Runoff could be diverted from the curb inlet to this bioretention that could be installed in the easement of this side of the road.
- Bioretention could be installed adjacent to the south bound lane at Dearborn Place. The bioretention could be located in the large easement south of the house overlooking Dearborn Place.
- There is a little triangle at corner of Dearborn and Ft Williams that could be used for a micro bioretention.

**Pros:**

- Large stormwater capture potential
- Slope of street makes capture easy at downstream end of street.
- Public Easement along Roads
- Large median

**Cons:**

- Impact to traffic flow during construction. However, this a low density residential so traffic impact could be minimized if construction is well timed.

## Duke Street.

**Duke St. between N Donelson St. and Fort Williams Pkwy**



**Flow Diversion Curb Cut in Median**



**Program Type:** Green Street

**GI Concepts:** Planters/Bioretention

**Field Notes:**

- A linear bioretention could be installed in the median on Duke St. between N Donelson St. and Fort Williams Pkwy.
- The median is located between the primary lanes of Duke St. and a side access lane.
- Both the main and side streets are crowned so that only the westbound lanes of the main street and the east bound lane of the side street drain towards the median.
- There is a curb cut in the median to convey runoff from the main street across the median to the side street where the runoff eventually goes to a curb inlet on the side street. This curb cut could be reconfigured to divert runoff into the median.

**Pros:**

- Large stormwater capture potential
- Slope of street toward the median makes runoff capture easy by setting up a couple curb cuts along the length of the street.
- Public Easement along Roads

**Cons:**

- Busy street may require coordination with other stake holders.
- Impact to traffic flow during construction.

**Wheeler Ave. (Dead End)****Wheeler Ave. (Dead End)**

Source: Google Maps Street View™

**Island Adjacent with Curb Inlet****Program Type:** Green Building/Parking**GI Concepts:** Green Roof, Bioretention, Tree islands, Porous Pavement**Field Notes:**

- This is light industrial land use with high imperviousness from buildings, loading bays and parking.
- Roof leader of the buildings are connected and provides an opportunity to disconnect to planters that could be installed adjacent to the buildings or installation of green roof.
- All tree/green islands could be converted to micro bioretention or planters to improve infiltrations of runoff.
- Observed good amount of unused impervious surfaces at the back of the building. These surfaces could be converted to green space or porous pavements.
- Also some of the impervious surfaces near curb inlets could be reclaimed for bioretention/planters.

**Pros:**

- Significant stormwater capture potential, especially from roof.
- Dead end of road with relatively low traffic. Parking areas will provide ample staging area during construction which will minimize disruption of traffic flow.



**Cons:**

- Would require coordination with private property owners.

**Wheeler Ave. at S Gordon Street****Wheeler Ave. (Dead End)****Island Adjacent with Curb Inlet****Program Type:** Green Building/Parking**GI Concepts:** Green Roof. Bioretention, Tree islands**Field Notes:**

- This is light industrial land use with high imperviousness from buildings, loading bays and parking. Roof leader of the buildings are connected and provides an opportunity to disconnect to planter that could be installed adjacent to the buildings. Roof has a lot of mechanical equipment which minimizes the potential for green roof implementation.
- Linear bioretention/planters could be installed in the median adjacent to the parking lot to improve infiltrations of runoff from parking lot.
- Existing trench drain that conveys runoff from parking lot to curb inlets near both entrances to the parking lot could be reconfigured to send the runoff to the median for infiltration.

**Pros:**

- Significant stormwater capture potential, especially from roof.
- Road with no through traffic relatively low traffic means less impact on traffic during construction.

**Cons:**

- Would require coordination with private property owner.
- Buildings may have less opportunity for green roof because of density of mechanical equipment on roof.

## 3830 Seminary Road (Beth El Hebrew Congregation)



Post Office Grounds



Exit to Post Office

**Program Type:** Green Parking,

**GI Concepts:** Bioretention, Porous Pavement, Planters,

**Field Notes:**

- Large parking lot without trees/green Island.
- Property slopes to the back and runoff from the property could be capture with a bioretention installed adjacent to the downstream end of the parking lot.

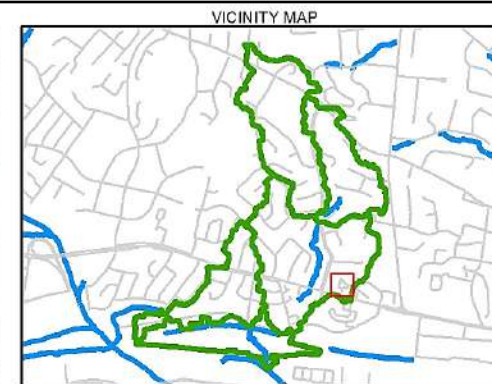
**Pros:**

- Large stormwater capture potential from parking lot.
- Ample area for staging during construction which helps minimize impact on traffic flow during construction.
- Good opportunity for community outreach through the church.

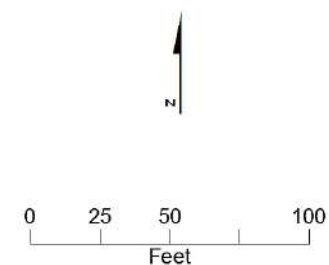
**Cons:**

- Would require coordination with private property owner.



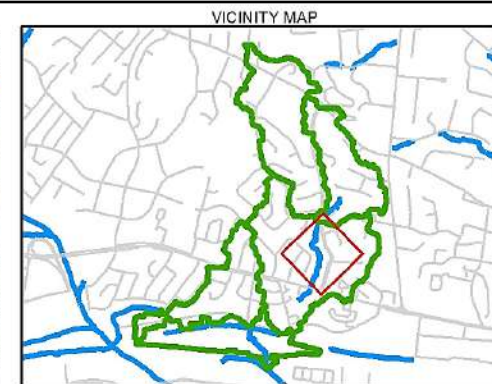
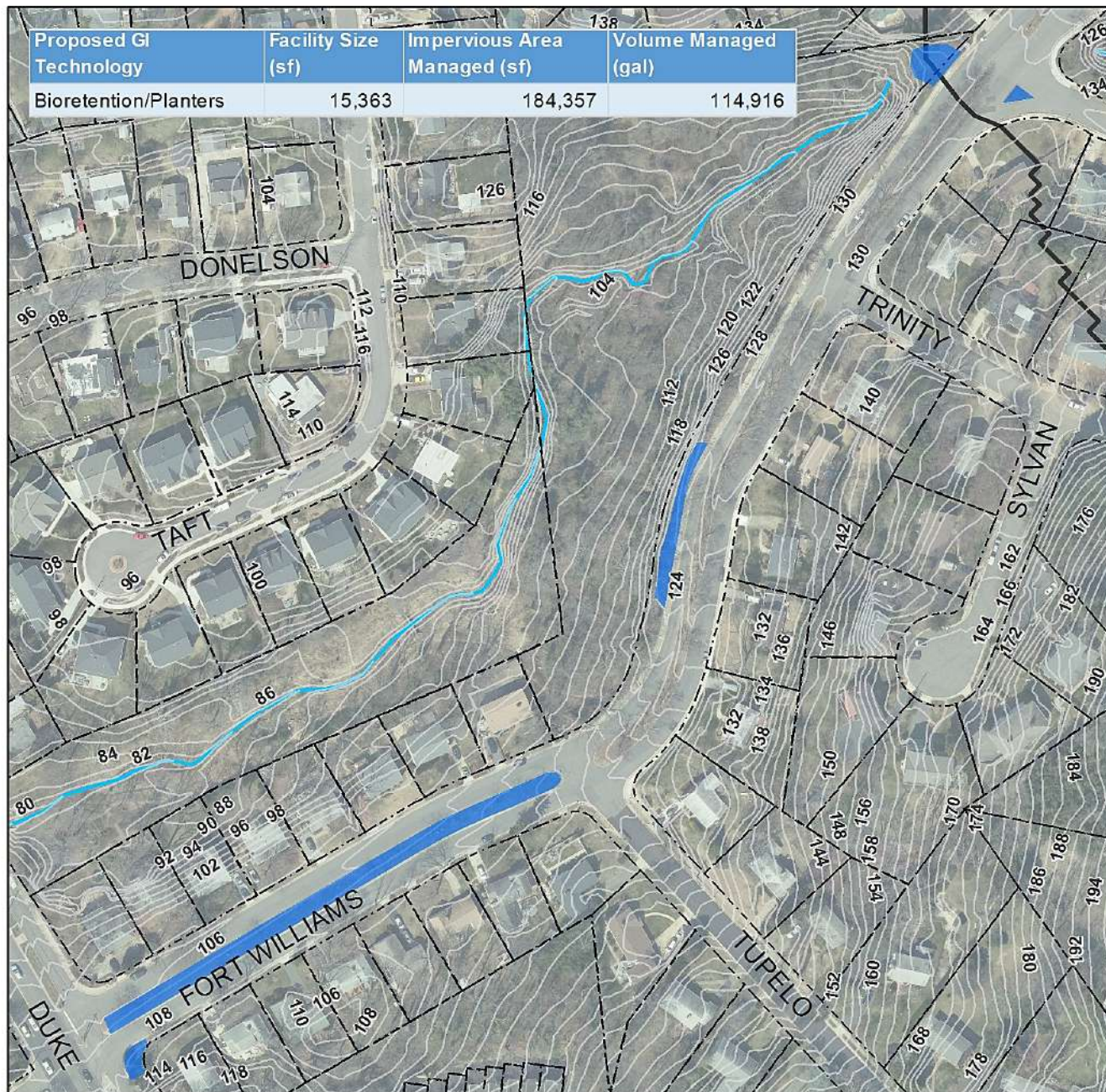


- LEGEND**
- Contours (ft)
  - Parcels
  - Watershed
  - City of Alexandria Streams
- Green Infrastructure Concepts**
- Bioretention/Planters
  - Cisterns
  - Green/Blue Roofs
  - Porous Pavement
  - Stream Daylighting
  - Surface Storage (Blue Streets)

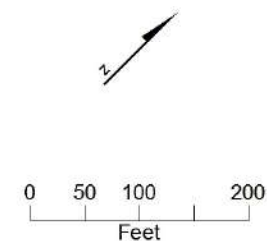


**Carriage House Circle**  
 Bioretention/Planters, Porous Pavement, Green Parking  
 Task 4 - Identify Problems and Develop Solutions  
 City of Alexandria Storm Sewer Capacity Analysis



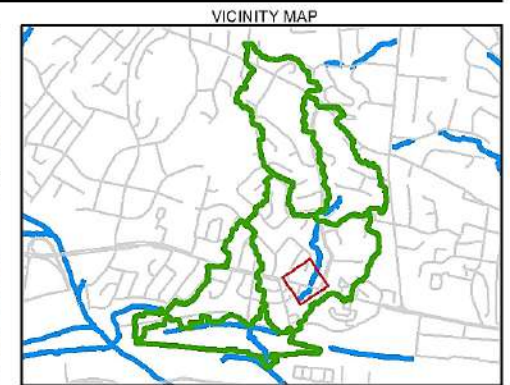
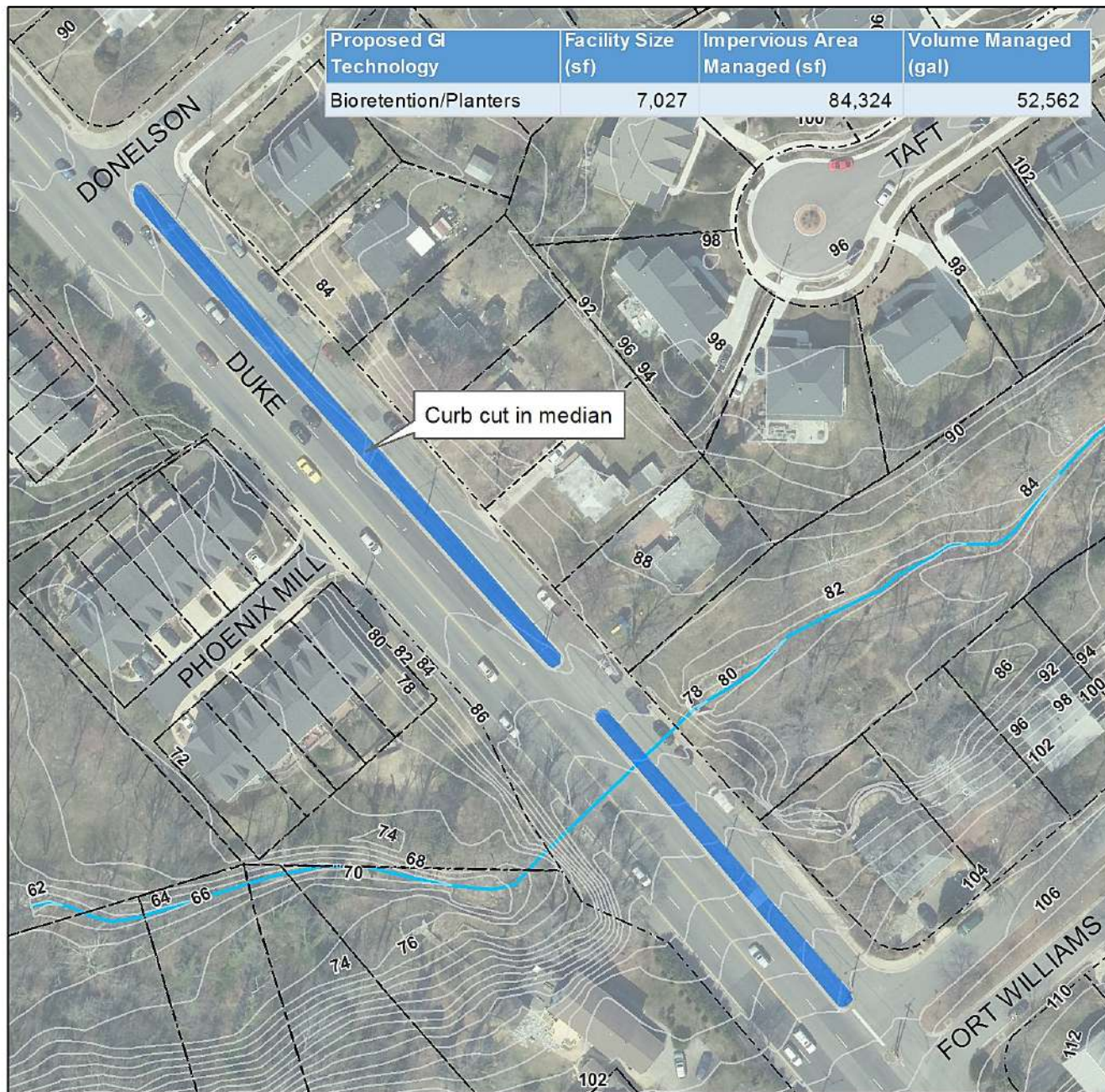


- LEGEND**
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  - Surface Storage (Blue Streets)

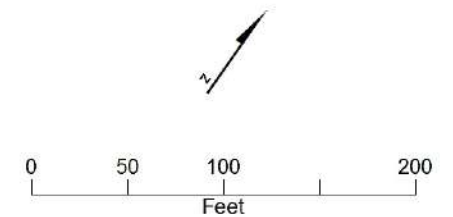


**Fort Williams Parkway**  
 Bioretention/Planters, Green Street  
 Task 4 - Identify Problems and Develop Solutions  
 City of Alexandria Storm Sewer Capacity Analysis





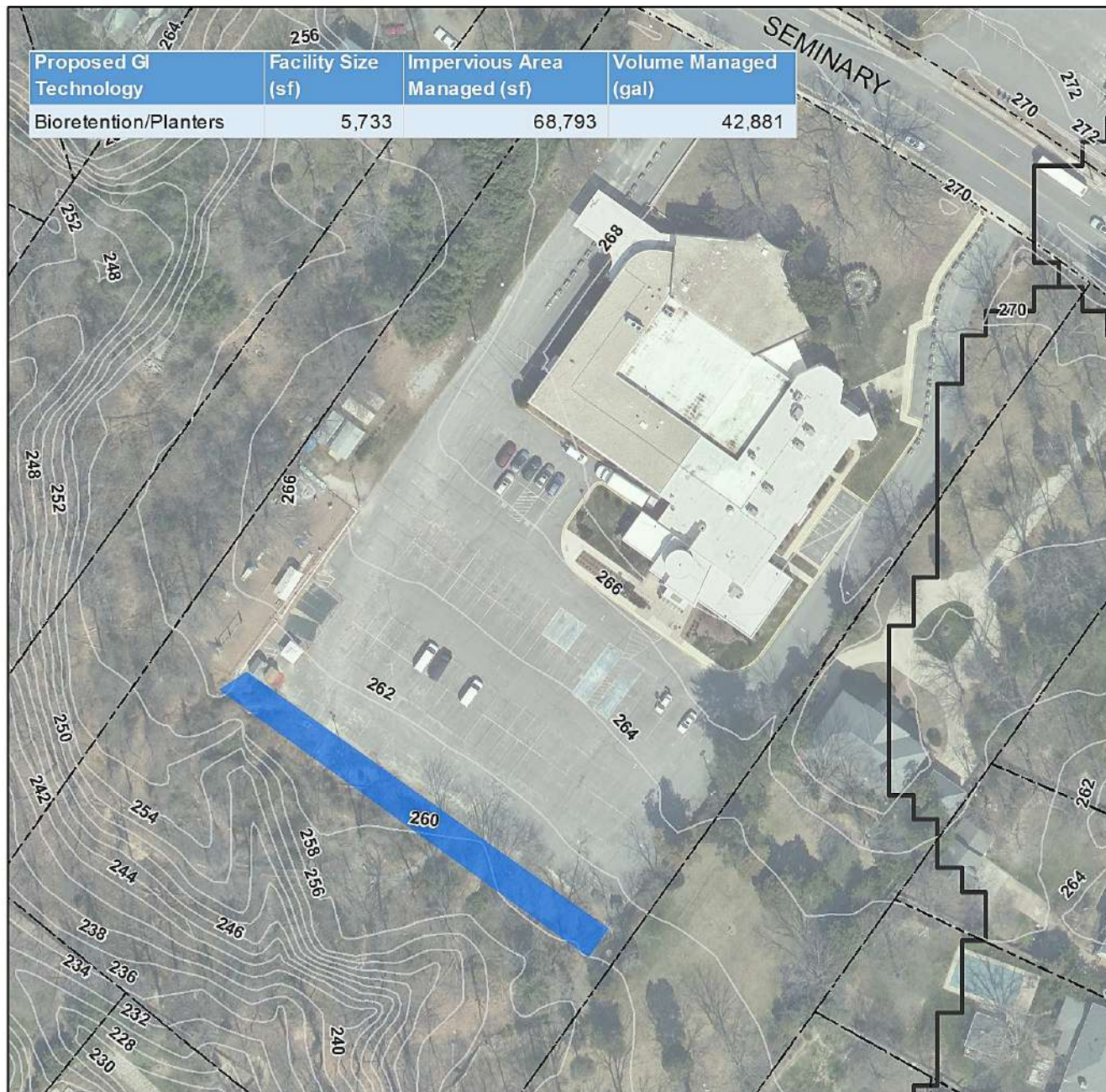
- LEGEND**
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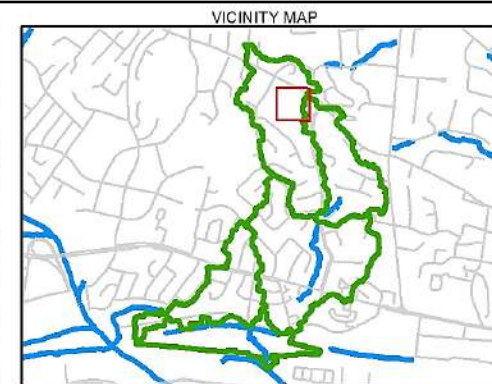
### Duke Street at North Donelson Street

Bioretention/Planters, Green Street  
Task 4 - Identify Problems and Develop Solutions  
City of Alexandria Storm Sewer Capacity Analysis

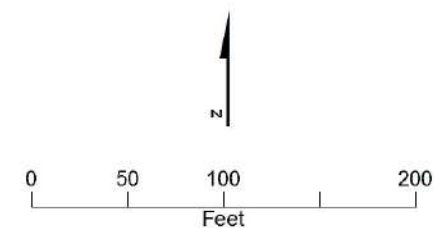




Proposed GI Technology	Facility Size (sf)	Impervious Area Managed (sf)	Volume Managed (gal)
Bioretention/Planters	5,733	68,793	42,881

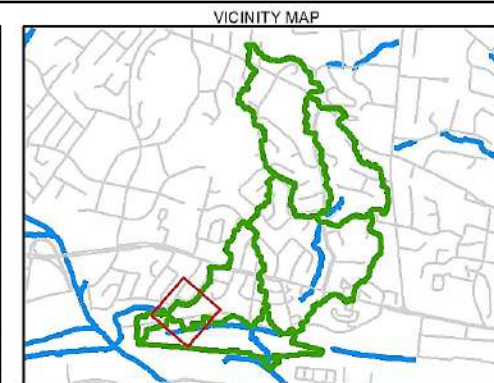


- LEGEND**
- Contours (ft)
  - ▭ Parcels
  - ▭ Watershed
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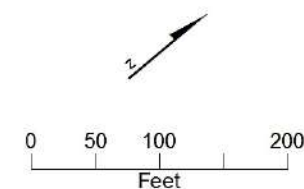


**3830 Seminary Road**  
**(Beth El Hebrew Congregation)**  
 Bioretention/Planters, Green Parking  
 Task 4 - Identify Problems and Develop Solutions  
 City of Alexandria Storm Sewer Capacity Analysis





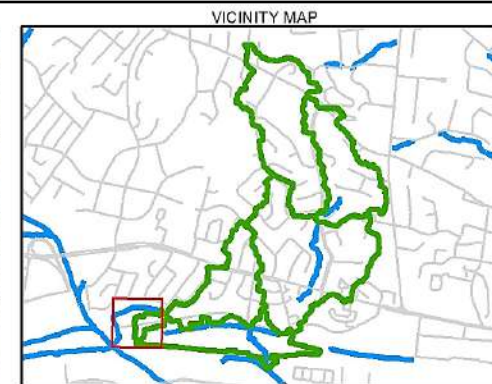
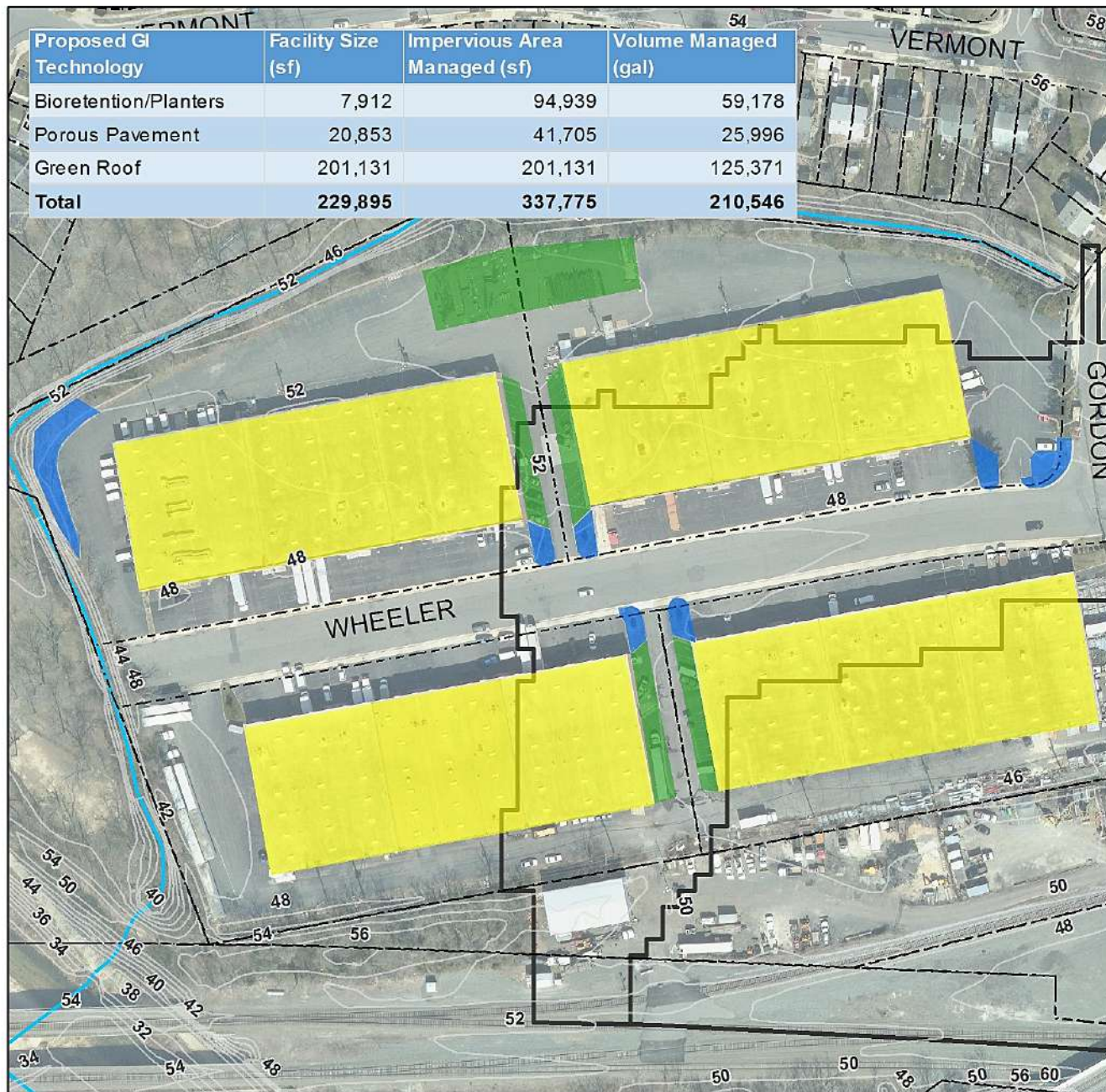
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  - Surface Storage (Blue Streets)



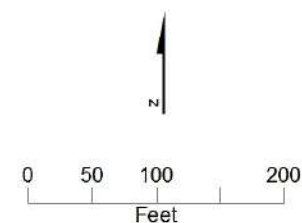
### Wheeler Avenue at South Gordon Street

Bioretention/Planters, Porous Pavement, Green Roof/  
Roof Disconnection, Green Building/Parking  
Task 4 - Identify Problems and Develop Solutions  
City of Alexandria Storm Sewer Capacity Analysis





- LEGEND**
- Contours (ft)
  - Parcels
  - Watershed
  - City of Alexandria Streams
- Green Infrastructure Concepts**
- Bioretention/Planters
  - Cisterns
  - Green/Blue Roofs
  - Porous Pavement
  - Stream Daylighting
  - Surface Storage (Blue Streets)



### Wheeler Avenue (Dead End)

Bioretention/Planters, Porous Pavement, Green Roof/  
Roof Disconnection, Green Building/Parking  
Task 4 - Identify Problems and Develop Solutions  
City of Alexandria Storm Sewer Capacity Analysis



## FACT SHEET: BIORETENTION AND STORMWATER PLANTERS



*Rain garden in a public park setting in Lancaster, PA*



*Right-of-way bioretention planting in Syracuse, NY*

Bioretention areas (often called Rain Gardens) are shallow surface depressions planted with specially selected native vegetation to treat and capture runoff and are sometimes underlain by sand or a gravel storage/infiltration bed. Bioretention is a method of managing stormwater by pooling water within a planting area and then allowing the water to infiltrate into the garden soils. In addition to managing runoff volume and mitigating peak discharge rates, this process filters suspended solids and related pollutants from stormwater runoff.

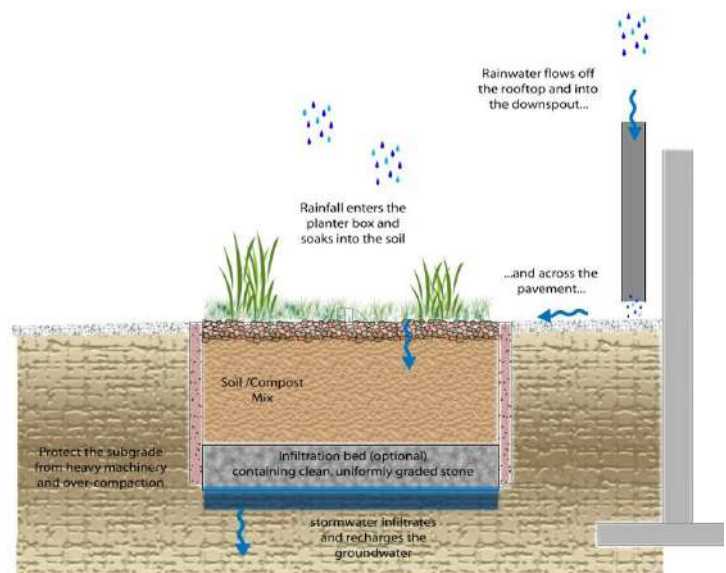
Bioretention can be designed into a landscape as a garden feature that helps to improve water quality while reducing runoff quantity. Rain Gardens can be integrated into a site with a high degree of flexibility and can balance nicely with other structural management systems including porous pavement parking lots, infiltration trenches, and non-structural stormwater BMPs. Bioretention areas typically require little maintenance once fully established and often replace areas that were intensively landscaped and required high maintenance.

A Stormwater Planter is a container or enclosed feature located either above ground or below ground, planted with vegetation that captures stormwater within the structure itself.

### BENEFITS

- Volume control & GW recharge, moderate peak rate control
- Versatile w/ broad applicability
- Enhanced site aesthetics and habitat
- Potential air quality & climate benefits

POTENTIAL APPLICATIONS	
Residential	Yes
Commercial	Yes
Ultra-Urban	Yes (Planters)
Industrial	Yes
Retrofit	Yes
Recreational	Yes
Public/Private	Yes



*Conceptual cross-section showing planter with infiltration*



## VARIATIONS

- Subsurface storage/infiltration bed
- Use of underdrain and/or impervious liner
- Planters – Contained (above ground), infiltration (below ground), flow-through
- Pre-treatment incorporated into design

## KEY DESIGN FEATURES

- Ponding depths 6 to 18 inches for drawdown within 48 hours
- Plant selection (native vegetation that is tolerant of hydrologic variability, salts, and environmental stress)
- Amended or engineered soil as needed
- Stable inflow/outflow conditions and positive overflow for extreme storm events
- Planters may require flow bypass during winter
- Planters - Captured runoff to drain out in 3 to 4 hours after storm even unless used for irrigation

## SITE FACTORS

- Water Table/ Bedrock Separation: 2-foot minimum, 4-foot recommended (N/A for contained planter)
- Soils: HSG A and B preferred; C & D may require an underdrain (N/A for contained planter)
- Feasibility on steeper slopes: medium
- Potential Hotspots: yes with pretreatment and/or impervious liner, yes for contained planter
- Maximum recommended drainage area loading: 15:1; not more than 1 acre to one rain garden

## MAINTENANCE

- Often requires watering during establishment
- Spot weeding, pruning, erosion repair, trash removal, mulch reapplication (as needed) required 2-3x/growing season
- Maintenance tasks and costs are similar to traditional landscaping

## COST

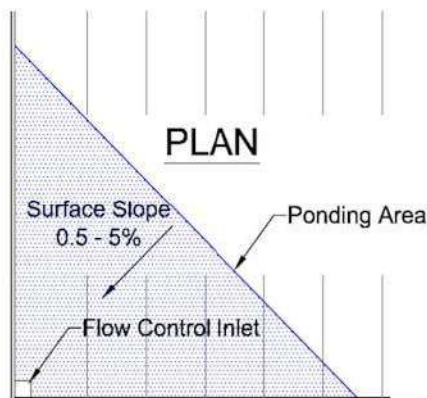
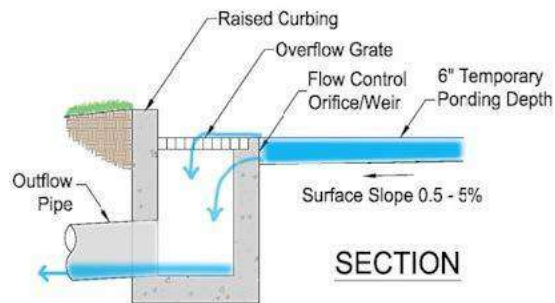
- Bioretention costs will vary depending on size/vegetation type/storage elements; typical costs \$10-25/ sq. ft.

## POTENTIAL LIMITATIONS

- Higher maintenance until vegetation is established
- Limited impervious drainage area to each BMP
- Requires careful selection & establishment of plants

STORMWATER QUANTITY FUNCTIONS		STORMWATER QUALITY FUNCTIONS		ADDITIONAL CONSIDERATIONS	
Volume	High	TSS	High	Capital Cost	Medium
Groundwater Recharge	High	TP	High	Maintenance	Low/Medium
Peak Rate	Medium	TN	Medium	Winter Performance	Medium
Erosion Reduction	Medium	Temperature	Medium/High	Fast Track Potential	Medium
Flood Protection	Medium			Aesthetics	High

## FACT SHEET: BLUE STREETS



### BENEFITS

- Reduces stress on drainage system
- Mitigates peak rate flow
- Cost-effective technique to manage stormwater
- Short duration storage
- Reduces need for subsurface excavation and construction

### POTENTIAL APPLICATIONS

Residential	Yes
Commercial	Yes
Ultra-Urban	Limited
Industrial	Yes
Retrofit	Yes
Highway/Road	Limited for Highway
Recreational	Yes
Public/Private	Yes/Yes

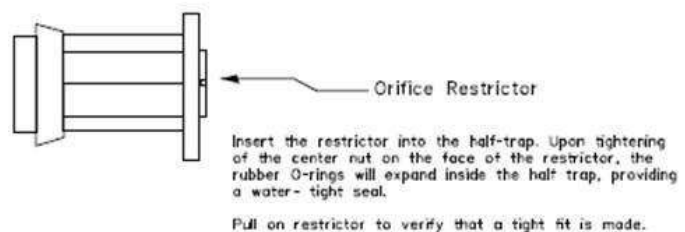
Blue streets refer to the practice of temporarily detaining stormwater, delaying its release and reducing its peak flow rate into the storm sewer system.

Surface storage practices have been used traditionally on rooftops (i.e. blue roofs) and in parking lots but can also be implemented in residential streets and right-of-ways with lower traffic volumes. These “blue streets” can be a cost-effective way to manage stormwater and address surcharging without significant subsurface excavation and construction interventions.

Surface storage is typically accomplished using drainage structures and retrofitting existing catch basins to feature devices such as orifice restrictors or vortex restrictors. Blue streets also emphasize minimizing the number of catch basins to the extent practical.

Blue streets (surface storage techniques) are often best implemented in alleys, low volume roads, and on private sites, for public perception and safety reasons.

## DRAINAGE STRUCTURES RESTRICTORS



*Drainage structure restrictors are key features of surface storage and blue streets. Source: City of Chicago design manual*

## VARIATIONS

- Flow control structures
- Orifice restrictors
- Vortex restrictors
- Reduction in number of catch basins/inlets on a street

## KEY DESIGN FEATURES

- Emergency overflows typically required
- Maximum ponding depths (less than one foot)
- Adequate surface slope to outlet
- Traffic volume, public safety, and user inconvenience must be taken into account

## SITE FACTORS

- Water table to bedrock depth – N/A
- Soils – N/A
- Slope – Requires relatively low slopes to provide appreciable storage
- Potential hotspots – yes
- Maximum drainage area – relatively small DA to individual inlets (similar to conventional inlets)

## MAINTENANCE

- Clean drainage structures and repair/replace parts as needed

## COST

- Drainage structures restrictors range in cost, for example installing a vortex restrictor can be approximately \$1000 per inlet

## POTENTIAL LIMITATIONS

- Not suitable for heavily-used roadways without adequate median/shoulder space
- Excess ponding on roadways may freeze in winter conditions
- Public safety perceptions and concerns
- Does not inherently address water quality and quantity – should generally be combined with other BMPs

STORMWATER QUANTITY FUNCTIONS		STORMWATER QUALITY FUNCTIONS		ADDITIONAL CONSIDERATIONS	
Volume	Low	TSS	Low	Capital Cost	Low
Groundwater Recharge	Low	TP	Low	Maintenance	Low/Medium
Peak Rate	Medium	TN	Low	Winter Performance	Medium
Erosion Reduction	Low	Temperature	Low	Fast Track Potential	High
Flood Protection	Medium			Aesthetics	Low

## FACT SHEET: CISTERNS/RAIN BARRELS



*Example of above-ground cistern with vegetation screening*

Cisterns (or rain barrels) are structures designed to intercept and store runoff from rooftops to allow for its reuse, reducing volume and overall water quality impairment. Stormwater is contained in the cistern structure and typically reused for irrigation or other water needs. This GI technology reduces potable water needs while also reducing stormwater discharges.

Cisterns can be located above or below ground and are containers or tanks with a larger storage capacity than a rain barrel, and often used to supplement grey water needs (i.e. toilet flushing) in a building, as well as irrigation. Rain barrels are above-ground structures connected to rooftop downspouts that collect rainwater and store it until needed for a specific use, such as landscape irrigation.

Cisterns and rain barrels can be used in suburban and urban areas where the need for supplemental onsite irrigation or other high water uses is especially apparent.

### BENEFITS

- Provides supplemental water supply
- Wide applicability
- Reduces potable water use
- Related cost savings and environmental benefits
- Reduces stormwater runoff impacts

POTENTIAL APPLICATIONS	
Residential	Yes
Commercial	Yes
Ultra-Urban	Yes, if demand exists
Industrial	Yes
Retrofit	Yes
Highway/Road	No
Recreational	Limited
Public/Private	Yes/Yes



*Rain barrel prototype example*

## VARIATIONS

- Cisterns – can be either underground and above ground
- Water storage tanks
- Storage beneath a usable surface using manufactured stormwater products (chambers, pipes, crates, etc.)
- Various sizes, materials, shapes, etc.

## KEY DESIGN FEATURES

- Small storm events are captured with most structures
- Provide overflow for large storms events
- Discharge/use water before next storm event
- Consider site topography, placing structure upgradient of plantings (if applicable) in order to eliminate pumping needs

## SITE FACTORS

- Water table to bedrock depth – N/A (although must be considered for subsurface systems)
- Soils – N/A
- Slope – N/A
- Potential hotspots – typically N/A for rooftop runoff
- Maximum drainage area – typically relatively small, based on storage capacity

## MAINTENANCE

- Use stored water and/or discharge before next storm event
- Clean annually and check for loose valves, leaks, etc. monthly during active season
- May require flow bypass valves or be taken offline during the winter

## COST

- Cisterns typically cost from \$3 to \$8/gallon/ Rain Barrels range from \$75 to \$300 each

## POTENTIAL LIMITATIONS

- Manages only relatively small storm events which requires additional management and use for the stored water.
- Typically requires additional management of runoff
- Requires a use for the stored water (irrigation, gray water, etc.)

STORMWATER QUANTITY FUNCTIONS		STORMWATER QUALITY FUNCTIONS		ADDITIONAL CONSIDERATIONS	
Volume	Low/Medium	TSS	Medium	Capital Cost	Medium
Groundwater Recharge*	Low/Medium	TP	Medium	Maintenance	Medium
Peak Rate*	Low	TN	Low	Winter Performance	Low
Erosion Reduction	Low	Temperature	Low	Fast Track Potential	Medium/High
Flood Protection*	Low			Aesthetics	Low/Medium

*\*Although stand-alone cisterns are expected to have lower benefits in these categories, if combined with downspout disconnection to landscaped areas the benefits can be increased significantly.*



## FACT SHEET: VEGETATED (GREEN) ROOFS AND BLUE ROOFS



Green roof (Philadelphia, PA)



Blue roof (NYC) / Photo – Gowanus Canal Conservancy

### BENEFITS

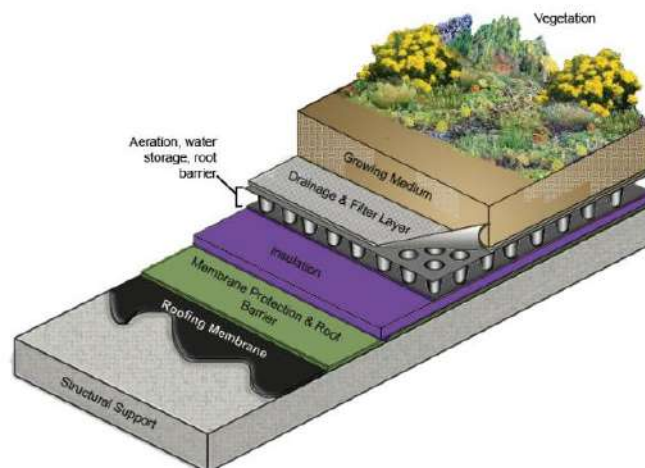
- High volume reduction (annual basis)
- Moderate ecological value and habitat (green roofs)
- High aesthetic value (green roofs)
- Energy benefits (heating/cooling)
- Urban heat island reduction

POTENTIAL APPLICATIONS	
Residential	Limited
Commercial	Yes
Ultra-Urban	Yes
Industrial	Yes
Retrofit	Yes
Highway/Road	No
Recreational	Limited
Public/Private	Yes/Yes

A green roof is a veneer of vegetation that is grown on and covers an otherwise conventional flat or pitched roof, endowing the roof with hydrologic characteristics that more closely match surface vegetation. The overall thickness of the veneer typically ranges from 2 to 6 inches and may contain multiple layers, such as waterproofing, synthetic insulation, non-soil engineered growth media, fabrics, and synthetic components. Vegetated roofs can be optimized to achieve water quantity and water quality benefits. Through the appropriate selection of materials, even thin vegetated covers can provide significant rainfall retention and detention functions.

Depending on the plant material and planned usage for the roof area, modern vegetated roofs can be categorized as systems that are intensive (usually > 6 inches of substrate), semi-intensive, or extensive (<4 inches). More maintenance, higher costs and more weight are the characteristics for the intensive system compared to that of the extensive vegetated roof.

Another GI rooftop technology - **Blue roofs** - are non-vegetated systems that employ stormwater control devices to temporarily store water on the rooftop and then release it into the drainage system at a relatively low flow rate. Storage can be provided by modifying roof drains or through the use of detention trays that sometimes have a lightweight gravel media. Blue roof and green roof technologies can also be combined in a design to achieve



Cross-section showing components of vegetated roof system



## VARIATIONS

- Green roofs - single media system, dual media system (with synthetic liner)
- Green roofs - Intensive, Extensive, or Semi-intensive

## KEY DESIGN FEATURES

- Engineered media should have a high mineral content and is typically 85% to 97% nonorganic.
- 2-6 inches of non-soil engineered media; assemblies that are 4 inches and deeper may include more than one type of engineered media.
- Irrigation is generally not required (or even desirable) for optimal stormwater management
- Internal building drainage, including provision to cover and protect deck drains or scuppers, must anticipate the need to manage large rainfall events without inundating the vegetated roof system.
- Assemblies planned for roofs with pitches steeper than 2:12 (9.5 degrees) must incorporate supplemental measures to insure stability against siding.
- The roof structure must be evaluated for compatibility with the maximum predicted dead and live loads. Typical dead loads for wet extensive vegetated covers range from about 12 to 36 pounds per square foot.
- Waterproofing must be resistant to biological and root attack. In many instances a supplemental root barrier-layer is installed to protect the primary waterproofing.
- Blue roofs: roof structure, waterproofing, accommodation for larger storm events/emergency overflows

## MAINTENANCE

- Once vegetation is fully established, little maintenance needed for the extensive system
- Maintenance cost is similar to native landscaping, \$0.10-\$0.35 per square foot
- Blue roof maintenance is similar to conventional roof maintenance (cleaning roof and drains as necessary)

## COST

- Green roofs: \$10 - \$35 per square foot, including all structural components, soil, and plants; more expensive than traditional roofs, but have longer lifespan; generally less expensive to install on new roof versus retrofit on existing roof
- Blue roofs: Typically add only \$1-\$5 per square foot compared to traditional roofs

## POTENTIAL LIMITATIONS

- Green roofs have higher maintenance needs until vegetation is established
- Need for adequate roof structure and waterproofing; can be challenging on retrofit application

STORMWATER QUANTITY FUNCTIONS*		STORMWATER QUALITY FUNCTIONS*		ADDITIONAL CONSIDERATIONS	
Volume	Medium/High	TSS	Low/Medium	Capital Cost	High
Groundwater Recharge	Low	TP	Low/Medium	Maintenance	Medium
Peak Rate	Medium	TN	Low	Winter Performance	Medium
Erosion Reduction	Low/Medium	Temperature	Medium	Fast Track Potential	Low
Flood Protection	Low/Medium			Aesthetics	High

\*For green roofs, blue roofs primarily function for peak rate control and flood protection.

## FACT SHEET: POROUS PAVEMENT



Porous (pervious) pavement is a Green Infrastructure (GI) technique that combines stormwater infiltration, storage, and a structural pavement consisting of a permeable surface underlain by a storage/infiltration bed. Porous pavement is well suited for parking areas, walking paths, sidewalks, playgrounds, plazas, basketball courts, and other similar uses.

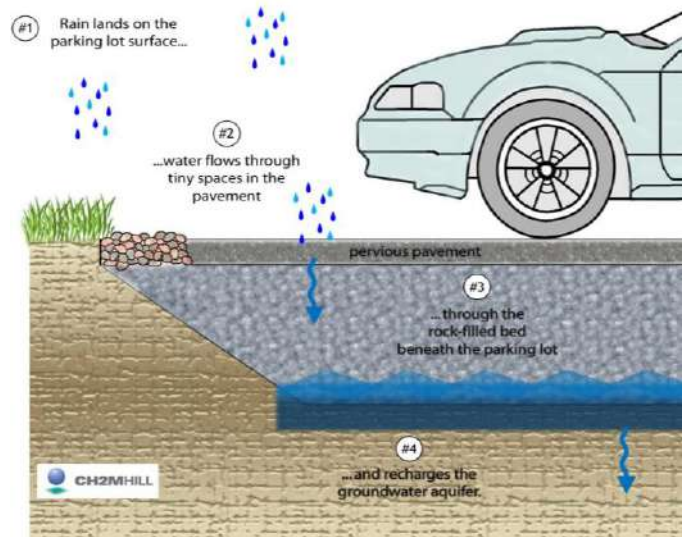
A porous pavement system consists of a pervious surface course underlain by a storage bed, typically placed on uncompacted subgrade to facilitate stormwater infiltration. The subsurface storage reservoir may consist of a stone bed of uniformly graded, clean and washed course aggregate with a void space of approximately 40% or other manufactured structural storage units. Porous pavement may be asphalt, concrete, permeable paver blocks, reinforced turf/gravel, or other emerging types of pavement.

### BENEFITS

- Volume control & GW recharge, moderate peak rate control
- Versatile with broad applicability
- Dual use for pavement structure and stormwater management
- Pavers come in range of sizes and colors
- Opportunity for public education/demonstration

### POTENTIAL APPLICATIONS

Residential	Yes
Commercial	Yes
Ultra Urban	Yes
Industrial	Limited
Retrofit	Yes
Highway	Limited
Recreational	Yes
Public/Private	Yes/Yes



Conceptual diagram showing how porous pavement functions

## KEY DESIGN FEATURES

- Soil testing required for infiltration designs
- Limit amount of adjacent areas that drain directly onto the surface of the porous pavement
- Uncompacted soil subgrade for infiltration
- Level storage bed bottoms
- Provide positive storm water overflow from bed
- Surface permeability greater than 20 inches per hour
- Secondary inflow mechanism recommended
- Pretreatment for sediment-laden runoff, limit sources of sediment/debris deposition

## SITE FACTORS

- Water Table/Bedrock Separation: 2-foot minimum
- Soils: HSG A&B preferred; HSG C&D may require underdrains
- Feasibility on steeper slopes: Low
- Potential Hotspots: Not without design of pretreatment system/impervious liner

## MAINTENANCE

- Clean inlets
- Vacuum biannually
- Maintain adjacent landscaping/planting beds
- Periodic replacement of aggregate in paver block joints (if applicable)
- Careful winter maintenance (no sand or other abrasives, careful plowing)

## COST

- Varies by porous pavement type
- Local quarry needed for stone filled infiltration bed
- Typically \$7-\$15 per square foot, including underground stormwater storage bed
- Generally more than standard pavement, but saves on cost of other BMPs and traditional drainage infrastructure

## POTENTIAL LIMITATIONS

- Careful design & construction required
- Pervious pavement not suitable for all uses/not suitable for steep slopes
- Higher maintenance needs than standard pavement

STORMWATER QUANTITY FUNCTIONS		STORMWATER QUALITY FUNCTIONS		ADDITIONAL CONSIDERATIONS	
Volume	High	TSS*	High	Capital Cost	Medium
Groundwater Recharge	High	TP	High	Maintenance	Medium
Peak Rate	Medium/High	TN	Medium	Winter Performance	Medium/High
Erosion Reduction	Medium/High	Temperature	High	Fast Track Potential	Low/Medium
Flood Protection	Medium/High			Aesthetics	Low to High

\* While porous pavements typically result in low TSS loads, sources of sediment should be minimized to reduce the risk of clogging.

## FACT SHEET: SOIL AMENDMENTS



*Healthy soils help vegetation thrive while also increasing soil infiltration rates Photo: S.Coronado*

Soil amendments can include a variety of practices that reduce the generation of runoff by improving vegetation growth, increasing water infiltration, and improving water holding capacity. For example, on existing turf grass, soil amendments can include placing a thin layer of compost or other materials and spreading them evenly over existing vegetation. Amendments on existing turf grass areas can be applied for several years to improve soil over time. Soil testing can indicate how many applications are appropriate. Existing grass areas can also be aerated to improve water transmission and allow for deeper incorporation of compost.

On new construction, redevelopment, and restoration projects, compost can be applied and deeply tilled into compacted soils to restore their porosity before the areas are re-vegetated (potentially with native landscaping, combining the benefits of both GI strategies).

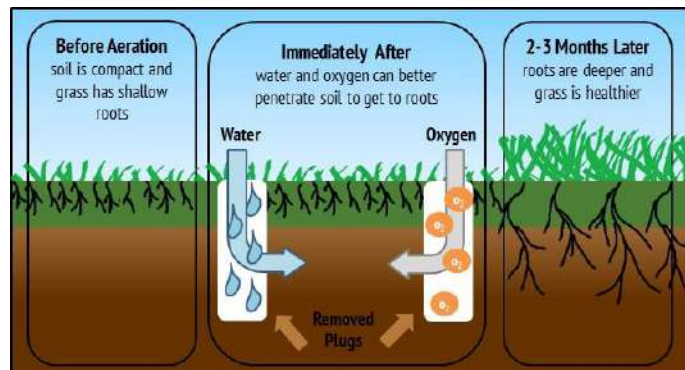
### BENEFITS

- Enhanced soil health and vegetation growth/root depth
- Improved soil infiltration rates
- Enhanced soil water holding capacity
- Reduced stormwater runoff from soil surface

POTENTIAL APPLICATIONS	
Residential	Yes
Commercial	Yes
Ultra-Urban	Limited
Industrial	Yes
Retrofit	Yes
Highway/Road	Yes
Recreational	Yes
Public/Private	Yes/Yes



*A variety of soil amendments are available depending on the specific soil conditions and desired result. Photo: Pahls Market*



*Physical aeration (tilling) can also help improve soil health and soil permeability/porosity. Image: GreenMaxLawns*

## VARIATIONS

- Treating turf grass or areas with more intensive plant palettes
- Combining amended soil areas with downspout disconnection
- Physical aeration/tilling of turf grass/vegetated areas can help to remedy soil compaction
- Compost, sand, microbes, mycorrhizae, gypsum, biochar, manure, worm castings, etc.
- Amendments can improve soil aggregation, increase porosity, and improve aeration and rooting depth

## KEY DESIGN FEATURES

- Soil bulk density and soil nutrient testing required
- Existing soil conditions should be evaluated before forming an amendment strategy

## SITE FACTORS

- Water table to bedrock depth – N/A
- Soils – Bulk density and nutrient levels
- Slope – Not recommended for use on slopes greater than 3:1
- Potential hotspots – N/A
- Maximum drainage area – N/A

## MAINTENANCE

- Replenishment of amendments on a regular basis may be required
- Aeration of soil often done at same time

## COST

- The cost of soil amendments ranges widely depending on the size and type. Larger projects are estimated to cost approximately \$5,000 per acre.

## POTENTIAL LIMITATIONS

- Viability depends upon soil testing results
- Certain types of soil may not be favorable for success with amendments
- Not a regulated industry – testing of amendment may be needed to ensure specifications
- Physical aeration should not be done near existing tree roots

STORMWATER QUANTITY FUNCTIONS		STORMWATER QUALITY FUNCTIONS		ADDITIONAL CONSIDERATIONS	
Volume	Medium	TSS*	Medium	Capital Cost	Low
Groundwater Recharge	Medium	TP*	Medium	Maintenance	Low/Medium
Peak Rate	Medium	TN*	Medium	Winter Performance	Medium
Erosion Reduction	High	Temperature	Low	Fast Track Potential	Medium
Flood Protection	Low/Medium			Aesthetics	Medium

\*Water quality benefits expected to vary widely depending on the condition of the soil/landscape prior to soil amendments.

## Appendix D

### Alternatives Analysis Results

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Appendix D - Alternative Analysis Summary

Tabulation of Solutions, Costs, and Scoring for Strawberry Run High-Priority Problem Areas

		Solution Summary			Flood Volume Summary					Weighted Solution Score								
Problem	Solution Technology	Project Name	Benefit-Cost Ratio	Existing Flood Volume (MG)	Solution Flood Volume (MG)	Flood Volume Reduction (MG)	Flood Volume Reduction (%)	Cost/Gallon of Flood Reduction (\$/gal)	Urban Drainage/ Flooding	Environmental Compliance	EcoCity Goals/ Sustainability	Social Benefits	Integrated Asset Management	City-Wide Maintenance Implications	Public			
Area ID	(Conveyance, Storage, Low GI, Medium GI, High GI)														Constructability	Acceptance	Total	
601	Conveyance	CONV-601	\$ 0.376	130.7	0.04	-	0.04	100%	\$ 9.87	17.1	0.0	0.0	0.0	6.6	16.2	4.3	4.8	49.1
601	Storage	STOR-601	\$ 0.157	173.7	0.04	-	0.04	100%	\$ 4.14	17.1	0.0	0.0	0.0	0.0	3.2	2.2	4.8	27.3
601	Low GI	LGI-601	\$ 0.038	1016.7	0.04	0.03	0.01	16%	\$ 6.28	2.7	2.6	2.5	2.0	6.6	13.0	4.3	4.8	38.4
601	Medium GI	MGI-601	\$ 0.200	244.8	0.04	0.02	0.02	47%	\$ 11.28	8.0	7.8	2.5	2.0	6.6	13.0	4.3	4.8	48.9
601	High GI	HGI-601	\$ 0.543	109.4	0.04	0.01	0.03	77%	\$ 18.59	13.2	13.1	2.5	2.0	6.6	13.0	4.3	4.8	59.5
602	Conveyance	CONV-602	\$ 0.729	55.3	0.02	-	0.02	100%	\$ 45.25	17.1	0.0	0.0	0.0	0.0	16.2	2.2	4.8	40.3
602	Storage	STOR-602	\$ 0.046	346.7	0.02	0.01	0.01	34%	\$ 8.38	5.9	0.0	0.0	0.0	0.0	3.2	2.2	4.8	16.1
602	Low GI	LGI-602	\$ 0.082	476.2	0.02	0.01	0.01	49%	\$ 10.31	8.5	2.0	3.6	2.9	0.0	13.0	4.3	4.8	39.1
602	Medium GI	MGI-602	\$ 0.434	117.7	0.02	0.00	0.02	96%	\$ 28.04	16.5	6.0	3.6	2.9	0.0	13.0	4.3	4.8	51.1
602	High GI	HGI-602	\$ 1.180	47.3	0.02	-	0.02	100%	\$ 73.19	17.1	10.1	3.6	2.9	0.0	13.0	4.3	4.8	55.8
603	Conveyance	CONV-603	\$ 0.345	135.9	0.02	-	0.02	100%	\$ 17.12	17.1	0.0	0.0	0.0	6.6	16.2	2.2	4.8	46.9
603	Storage	STOR-603	\$ 0.378	72.4	0.02	-	0.02	100%	\$ 18.74	17.1	0.0	0.0	0.0	0.0	3.2	2.2	4.8	27.3
603	Low GI	LGI-603	\$ 0.065	618.0	0.02	0.02	0.00	17%	\$ 19.24	2.9	2.2	3.5	2.8	6.6	13.0	4.3	4.8	40.1
603	Medium GI	MGI-603	\$ 0.343	161.0	0.02	0.00	0.02	79%	\$ 21.56	13.5	6.7	3.5	2.8	6.6	13.0	4.3	4.8	55.2
603	High GI	HGI-603	\$ 0.932	68.0	0.02	-	0.02	100%	\$ 46.22	17.1	11.3	3.5	2.8	6.6	13.0	4.3	4.8	63.4



## Appendix E

### Basis of Cost

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# City of Alexandria Storm Sewer Capacity Analysis

## Planning Level Cost Information

PREPARED FOR: City of Alexandria Transportation  
and Engineering Services

COPY TO: File

PREPARED BY: CH2M HILL

DATE: May 15, 2014

PROJECT NUMBER: 240027

## Introduction

The City of Alexandria, Virginia, has experienced repeated and increasingly frequent flooding events attributable to old infrastructure, inconsistent design criteria, and perhaps climate change. The purpose of the stormwater capacity analysis project is to provide a program for analyzing storm sewer capacity issues, identifying problem areas, developing and prioritizing solutions, and providing support for public outreach and education. The project is being implemented in phases by watershed. The watersheds include Hooffs Run, Four Mile Run, Holmes Run, Cameron Run, Taylor Run, Strawberry Run, Potomac River, and Backlick Run.

This technical memorandum provides details on the basis of cost estimates developed for each solution and the watershed wide alternatives. The information includes planning level unit cost for conveyance, storage and green infrastructure solutions.

These cost estimates are considered a Class 4 - Planning Level estimate as defined by the American Association of Cost Engineering (AACE), International Recommended Practice No. 18R-97, and as designated in ASTM E 2516-06. It is considered accurate to +50% to -30% based up to a 15% complete project definition.

## Definitions

The following cost terminologies are used within this technical memorandum:

- **Construction cost:** Installed cost, including materials, labor, and site adjustment factors such as overcoming utility conflicts, dewatering, and pavement restoration.
- **ENRCCI Cost Adjustment Factor:** Cost adjustment factor of 0.9 to adjust cost to October 2013 dollars for the DC-Baltimore metro area
- **Service and Contingency Factor (SCF)** A factor of 1.4 is applied for this project to account for engineering and design expenses (20%) and for contingency allowance (20%).
- **Capital cost:** Construction cost multiplied by a Service and Contingency Factor (SCF) to cover engineering and design and contingency allowance.
- **Operating cost:** Operation and maintenance were not considered for this project.

## Gravity Sewer Relief Costs

Conveyance projects were costed on a per linear foot basis, based on pipe size and depth. The construction cost rates (\$/ft) for gravity sewer replacement are listed in Table 1. Cost rates are shown for different road types. The Gravity sewer cost rates include complete installation of sewer pipes, inlets/manholes, and other ancillary structures as well as surface restoration. The costs were established through literature review and updated based on an assessment of bid tabulation data from Kansas City metro area between 2008 and 2012, and a comparison to Fairfax County, VA unit cost schedule, March 2013. All costs were adjusted to Washington DC, 2013 dollars using Engineering News-Record Construction Cost Index (ENRCCI) adjustment factors.

Factors are applied to the construction cost of gravity sewer pipe replacement to reflect the cost associated with crossing under streams and railroads as listed in Table 2.

Costs of routine O&M, inspection and cleaning at periodic intervals during the life of the gravity sewer were assumed to part of City-wide facilities maintenance plan and should take place even though those costs are not specifically included here.

TABLE 1  
**Open Cut Gravity Sewer Construction Costs**

Pipe Diameter (in)	Material	Sewer Construction Cost (\$/LF) <sup>(1)</sup>					
		Trench depth up to 10 feet		Trench depth 10 to 15 feet		Trench depth 15 to 20 feet	
		Residential	Arterial	Residential	Arterial	Residential	Arterial
8	PVC	\$90	\$104	\$113	\$130	\$140	\$162
10	PVC	\$113	\$131	\$140	\$163	\$176	\$204
12	PVC	\$122	\$140	\$152	\$175	\$190	\$218
15	PVC	\$131	\$153	\$163	\$192	\$204	\$239
18	PVC	\$140	\$162	\$175	\$203	\$218	\$253
21	PVC	\$162	\$189	\$203	\$237	\$253	\$295
24	PVC	\$185	\$212	\$230	\$265	\$288	\$330
30	RCP	\$257	\$297	\$320	\$372	\$401	\$464
36	RCP	\$306	\$356	\$383	\$445	\$478	\$555
42	RCP	\$360	\$414	\$450	\$518	\$563	\$647
48	RCP	\$410	\$473	\$512	\$590	\$640	\$738
54	RCP	\$459	\$531	\$574	\$664	\$717	\$830
60	RCP	\$509	\$585	\$635	\$732	\$795	\$914
72	RCP	\$815	\$936	\$1,018	\$1,170	\$1,273	\$1,463

(1) Listed construction costs have been adjusted to October 2013 dollars using ENRCCI for the DC-Baltimore Metro area.

TABLE 2  
Gravity Pipe Construction Cost Factors

Type of Crossing	Cost Factor
Stream	3
Railroad	7

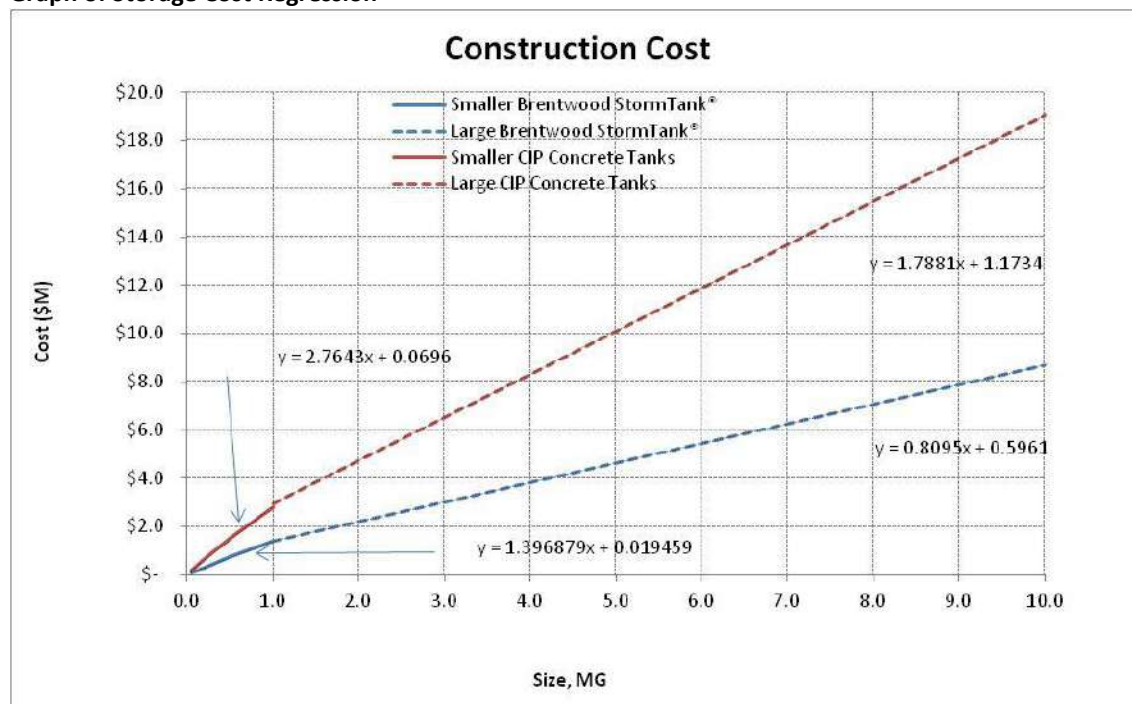
## Storage Facility Cost Information

Cost estimates for the storage facilities were developed for two technologies: A traditional underground cast-in-place concrete tank and an alternative stackable modular unit installed underground and wrapped with an impermeable or permeable liner.

The CIP Concrete storage facility construction cost was developed as a customized cost estimate based on CH2M HILL's Program Alternative Cost Calculator (PACC) Tool. The costs are construction costs only and do not include administration costs, engineering costs, contingencies, and other soft costs. The costs for smaller storage units with volumes less than 1 million gallon were found to be high for the CIP concrete tank. Hence, a separate takeoff cost estimate was developed for smaller storage volume; less than 1 million gallons.

A separate cost estimate was developed for the stackable modular units. There is an increasing use of these technologies in the industry and the cost of installation is getting increasingly competitive compared to traditional storage methods. Construction costs were developed based on one such stackable modular unit, StormTank® modules by Brentwood Industries. The cost for the Brentwood StormTank® modules came out significantly less than that for CIP concrete tanks. For the purpose of the evaluation of watershed wide alternative solutions, the StormTank® modules was used as the most cost effective alternative, however site specific conditions will determine which technology will be most appropriate in a given location. For example a site with high water table may make the use of CIP concrete tanks preferable over the StormTank® modules. The estimated construction costs for the CIP concrete tanks and the Brentwood StormTank® are provided in Figure 1.

FIGURE 1  
Graph of Storage Cost Regression



The following assumptions were made for storage tank selection and sizing:

1. Offline enclosed underground storage will be active only during wet weather events.
2. Options for odor control were not considered.
3. Costs for storage facilities with intermediate storage volumes were interpolated based on linear regression shown in Figure 1.

## Green Infrastructure (GI) Cost Information

A variety of sources and professional judgment were used to develop the GI costs. Where technologies were directly comparable, costs were updated based on Fairfax County, VA unit cost schedule, March 2013. The unit costs used to develop GI implementation cost are included in Table 4. Costs reflecting stand-alone projects (e.g., installing a green roof on top of an existing building) were used for costing alternatives solutions. Incremental costs of adding GI to an existing project can provide significant savings and are provided for reference, but not used directly in cost estimates for this project.

In the CASSCA Project GI is being proposed as a series of GI programs applicable to specific land uses (e.g. green parking is applicable to parking lots). Each GI program may consist of multiple GI technologies which drive the cost of implementing that program. Table 5 lists and the relative amounts of area designated for the GI technologies assumed to be part of each GI program and the resultant unit cost for each GI program.

TABLE 4  
Unit Construction Costs of Green Infrastructure Technologies

Green Technology	Stand Alone Cost Proposed for GI Plan (\$/GI acre)	Loading Ratio (Ratio of Area Managed to Area of GI)	Stand-Alone Cost Proposed for GI Plan (\$/acre managed)	Incremental GI Cost Compared to Stand-Alone
Native Landscaping/Soil Amend.	\$ 5,000	1	\$ 5,000	50%
Rain Barrels <sup>1</sup> and Native Landscaping/Soil Amend.	\$ -	N/A	\$ 15,000	90%
Cisterns <sup>2</sup>	N/A	N/A	\$ 34,000	90%
Blue Street/Inlet control devices	N/A	N/A	\$ 22,500	N/A
Rain Gardens	\$ 436,000	12	\$ 36,000	70%
Stormwater Trees <sup>3</sup>	\$ 34,700	0.5	\$ 69,000	50%
Bioswale/Bioretenention	\$ 1,045,000	12	\$ 87,000	70%
Porous Pavement/ Infiltration Trench	\$ 436,000	4	\$ 109,000	70%
Green Roof <sup>4</sup>	\$ 501,000	1	\$ 501,000	43%

<sup>1</sup> Each rain barrel is assumed to manage 350 ft<sup>2</sup> of rooftop; therefore, 124.5 barrels are required for 1 acre of roof.

<sup>2</sup> Each 1000-gallon cistern is assumed to manage 6,500 ft<sup>2</sup> of impervious area; therefore, 6.7 barrels are required for 1 acre.

<sup>3</sup> Trees are assumed to have an average 10-foot canopy radius (314 ft<sup>2</sup>), with 50 percent assumed to be overhanging impervious area.

<sup>4</sup> Incremental cost of green roofs set to 43 percent to match the District's \$5/ ft<sup>2</sup> (\$217,800/acre) green roof incentive program.

TABLE 5

**Green Infrastructure Technology Elements and Unit Construction Cost of Each Green Program**

Green Technology	% Area of Program Assigned to Each GI Technology						
	Blue Streets	Green Alley	Green Buildings	Green Parking	Green Roofs	Green Schools	Green Schools
Native Landscaping/Soil Amend.	-	-		-	-	-	-
Rain Barrels <sup>1</sup> and Native Landscaping/Soil Amend.	-	-	30%	-	-	-	-
Cisterns	-	-	10%	-	-	-	-
Blue Street/Inlet control devices	100%					-	-
Rain Gardens	-	-	30%	-	-	-	-
Stormwater Trees	-	-		-	-	-	30%
Bioswale/Bioretenention	-	-	30%	50%	-	65%	30%
Porous Pavement/ Infiltration Trench	-	100%		50%	-	30%	40%
Green Roof	-	-	-	-	100%	5%	-
<b>Unit Cost (\$/acre managed)</b>	<b>\$22,500</b>	<b>\$109,000</b>	<b>\$44,800</b>	<b>\$98,000</b>	<b>\$501,000</b>	<b>\$114,300</b>	<b>\$90,400</b>

Three levels of green infrastructure implementation were evaluated for this project:

- High Implementation – Manage 50% of total impervious area in the shed
- Medium Implementation – Manage 30% of total impervious area in the shed
- Low Implementation – Manage 10% of total impervious area in the shed

The unit cost of implementing GI at the various implementation levels is driven by the availability of GI opportunity areas. As the area available to achieve a GI implementation level become scarce, the cost to achieve that level on GI implementation also increases. It was assumed that GI implementation would focus, in succession, from the most to the least cost effective programs and technologies. That is, for each level of GI implementation the most cost effective program and technologies would be implemented first until the available opportunities for those programs are exhausted. If the level of implementation is not achieved with the most cost effective program, the next most cost effective program is considered in that order until the desired level of GI implementation is achieved. Therefore Low Implementation would be more cost effective (lower cost per acre managed). The unit cost for each implementation level was computed separately for each watershed based on the cost information presented above and the distribution of areas available for GI implementation.

## Green Opportunities

Opportunities for blue streets, green streets and alleys, green buildings, green parking, green roofs, and green schools were identified by completing a desktop analysis using the City's 2011 basemap data, including:

- Roads (Road\_y and Road\_lc)
- Buildings (Blds\_y)
- Parking lots (Parking\_y)
- Zoning (Zoning\_y)
- Parcels (Parcels\_y)



The approach to identifying potential opportunities for each program is provided below. All opportunities were combined into a single shapefile of polygons with an attribute for area calculated in acres.

### **Blue Streets**

Local or Residential roads with an average slope less than or equal to 1% and a maximum slope less than or equal to 3%. Road slope was estimated using ArcGIS 3D Analyst tools and the Road\_Lc feature and City of Alexandria DEM as inputs.

### **Green Streets and Alleys**

Green streets and alleys were identified using the Road\_Lc and Road\_y features to identify roads classed as Arterial, Primary Collector, Residential Collector, Local, and Alley with an average slope less than or equal to 5%. Roadways that fall within school parcels were removed from this layer because they are included in the Green Schools program. Road slope was estimated using ArcGIS 3D analyst tools and the Road\_Lc feature and City of Alexandria DEM as inputs.

### **Green Buildings**

Green buildings opportunities include buildings where disconnection may be possible. Based on a windshield survey of Taylor Run, approximately 50% of residential buildings, not including single family detached homes, may have opportunities for downspout disconnection. To identify these opportunities, buildings with a BUSE of '1-Residential' were selected from the Blds\_y features to identify all residential buildings. This selection was narrowed to apartment buildings and larger residential developments, removing detached houses (BTYPE = 'Detached house'), buildings with less than 5 units (BUNITS < 5), as well as removing nursing homes, hotels, and detention centers. Residential buildings on school properties were also removed because those are accounted for in the Green Schools program. Buildings with a footprint greater than 20,000 square feet were also removed because these buildings are likely too large for a disconnection program.

The footprint of the final selection was reduced by approximately 50% (based on the result of the Taylor Run windshield survey) to approximate the total area of impervious surfaces that could potentially be managed through a disconnection program.

### **Green Parking**

Green parking opportunities were identified as parking lots in the Parking\_y feature class with a parking area over 3,000 square feet. Parking lots on school parcels were removed from this selection because they are accounted for in the Green Schools program.

### **Green Roofs**

Green roof opportunities were identified by selecting buildings in the Blds\_y feature class with a footprint over 20,000 ft<sup>2</sup> that have a building use (BUSE) of Commercial, Industrial, Institution, Transportation, and Multiple or Mixed use. Also included were buildings over 20,000 ft<sup>2</sup> that were within a Commercial, Industrial, Coordinated Development District, or Mixed Use zone based on the Zoning\_y feature class, unless those buildings were garage/sheds. Buildings on school parcels were removed from this selection because they are accounted for in the Green Schools program.

### **Green Schools**

School parcels were identified by selecting all parcels with a land description (LANDDESC) of 'ED. PUBLIC SCHOOLS', 'PRIVATE ED ENSTS.', or 'ST. ED. INSTITUTIONS' or with an owner name or address that indicated it was school property. School buildings with potential for green roofs were identified by selecting all buildings on school parcels or buildings in the Blds\_y features with the word 'school' in the building name (BNAME) or building campus (BCAMPUS) fields where the footprint is over 3,000 ft<sup>2</sup>. All remaining impervious surfaces on the school parcels (roads, sidewalks, small buildings, recreation facilities, etc.) were identified as opportunities for green schools.